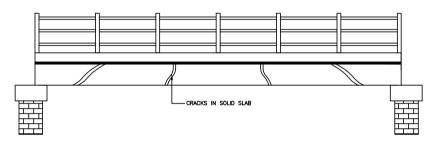
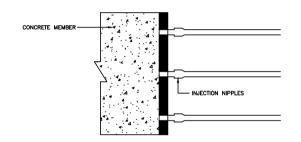


CRACKS IN GIRDER OF T-BEAM DECK SLAB

SEQUENCE OF OPERATION FOR SEALING OF CONCRETE CRACKS





CRACKS IN R.C.C. SOLID SLAB

CROSS SECTION A-A OF CRACK MEMBER

TYPICAL METHODOLOGY

(DETAILS AS PROVIDED IN TECHNICAL SPECIFICATIONS)

- 1. SURFACE ADJACENT TO CRACK AND AREA OF APPLICATION SHALL BE CLEANED OF DUST. DIRT. BREASE OIL, EFFLORESANCE AND OTHER FOREIGN MATERIAL BY BRUSHING/WATER JETTING OR SAND BLASTING.
- 2. PROVIDE ENTRY PORTS ALONG CRACK AT 300 MM C/C OR THICKNESS OF MEMBER WHICHEVER IS DRILLED HOLE 13 mm DIA AND 200 mm DEEP DEPTH SHALL NOT BE MORE THAN 60 % OF DEPTH OF MEMBER.
- 3. SEAL THE CRACKS WITH SURFACE SEAL MATERIAL BETWEEN ENTRY PORTS.

- INJECT EPOXY ADHESIVE FROM LOWEST ENTRY PORT INJECT TILL ADHESIVE APPEAR AT THE NEXT ENTRY PORT.
- 5. REPEAT THE PROCESS OF INJECTING EPOXY FROM NEXT ENTRY PORT UNTIL THE CRACK ARE COMPLETELY FILLED.
- 6. IF PORT TO PORT TRAVEL OF EPOXY IS NOTICED OR THE VOLUME OF THE INJECTED MATERIAL EXCEED 2 LITER AT A PARTICULAR ENTRY PORT WORK SHALL BE STOPPED AND THE ENGINEER SHALL BE CONSULT.
- SEALING OF CRACK BY INJECTION OF EPOXY SHALL BE CARRIED OUT AS PER MORTH SPECIFICATION 2803

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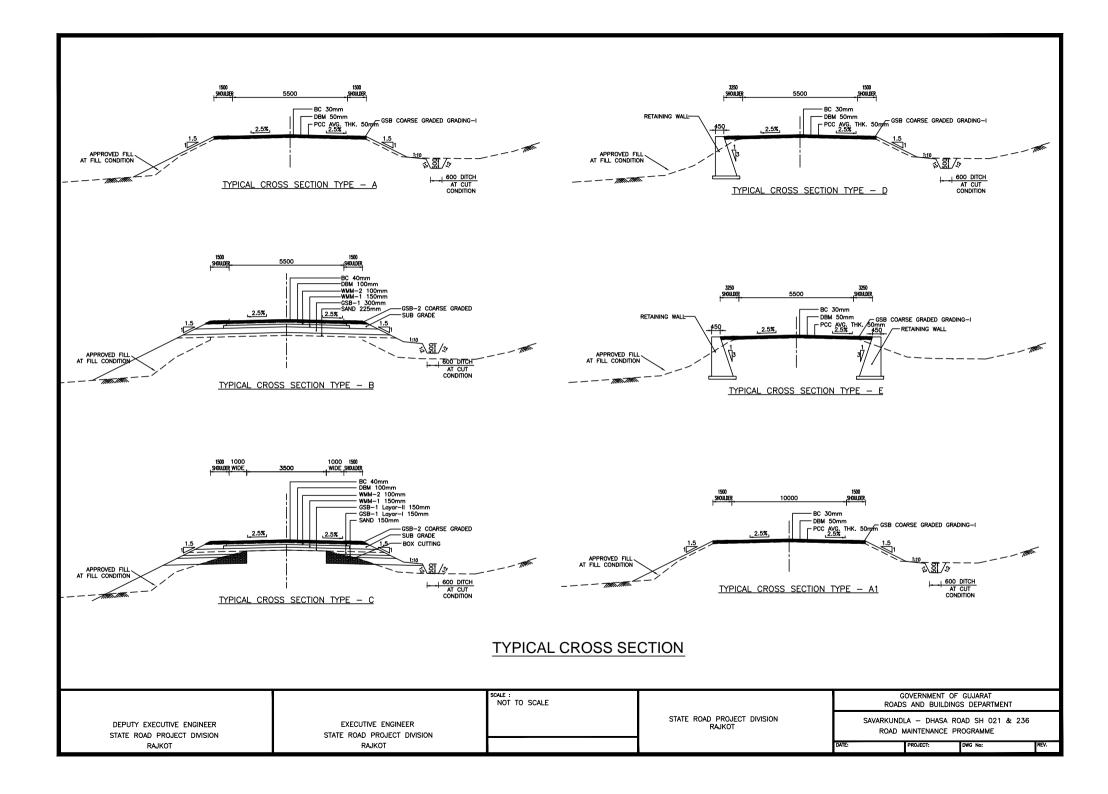
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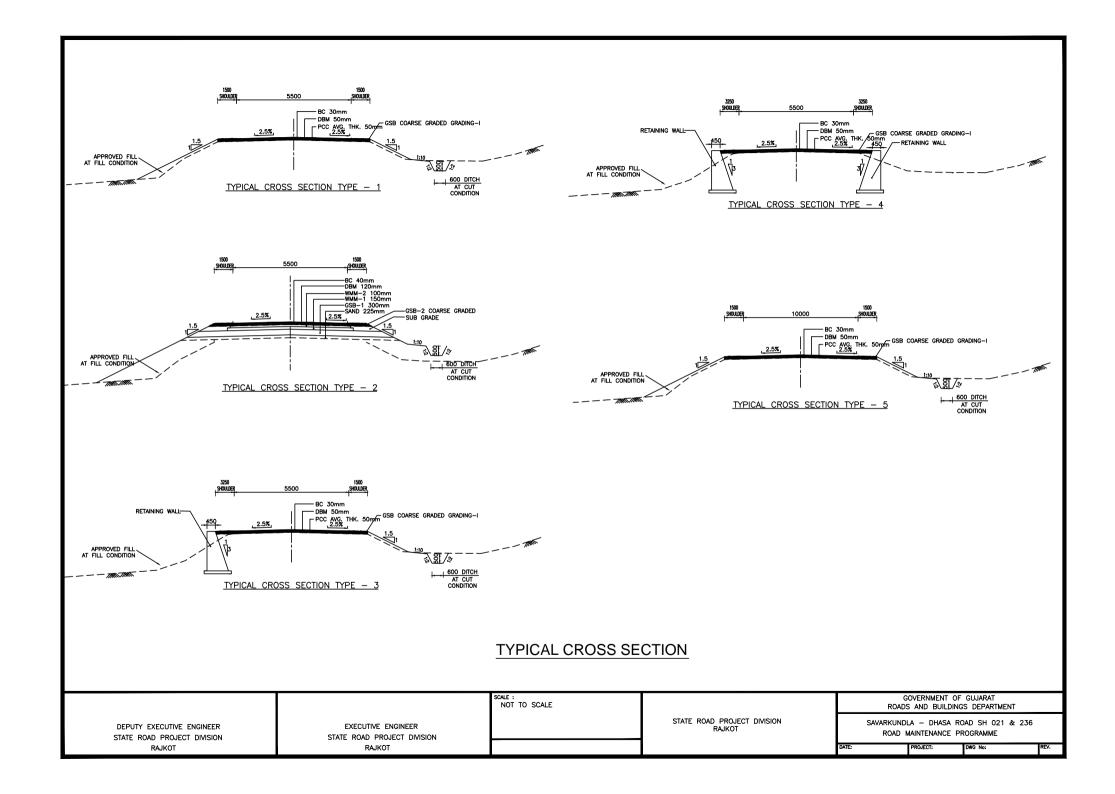
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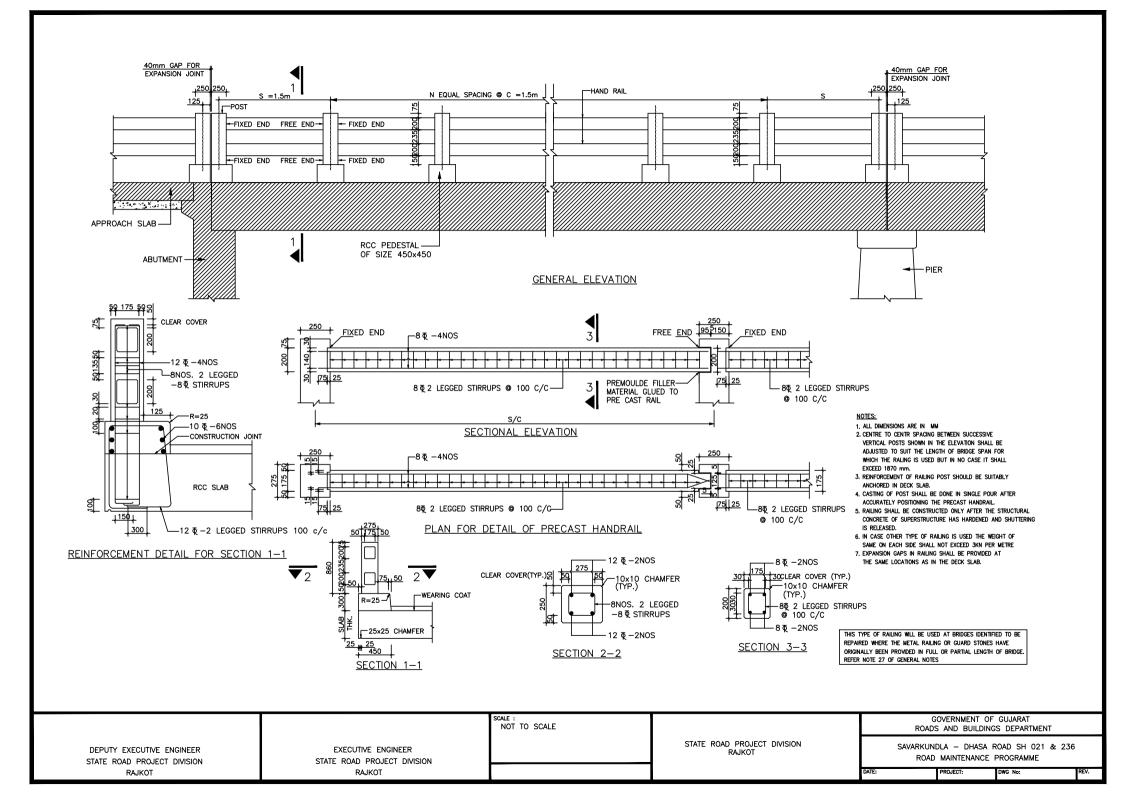
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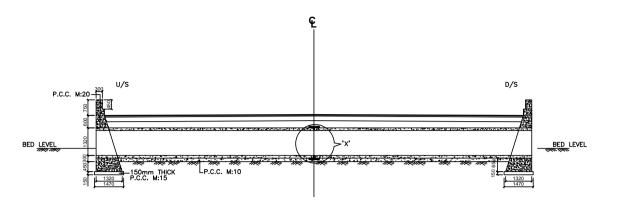
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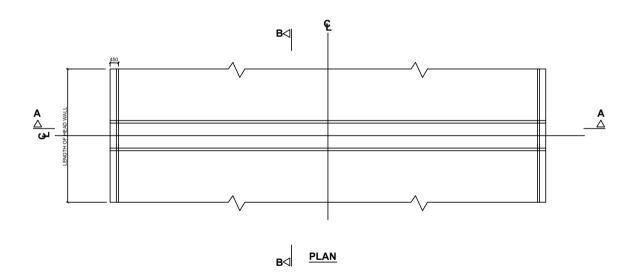




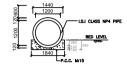




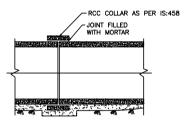
SECTION A-A



TYPICAL GAD FOR SINGLE PIPE CULVERT



SECTION B-B



DETAILS OF 'X'

NOTES:

- 1. THIS DRAWING HAS TO BE READ IN CONJUNCTION WITH PPWCS/BR/SD/101 TO 103
- 2. THIS DRAWING IS VALID ONLY FOR FIRST CLASS BEDDING CAN BE USED FOR MAX. HEIGHT OF FILLING 4m.
- 3. PIPES SHOULD CONFORM TO IS :-458.
- 4. LONGITUDINAL SLOPE OF PIPE SHOULD BE MINIMUM OF 1:1000
- ALL DIMENSIONS IN MILLIMETERS EXCEPT WHERE OTHERWISE MENTIONED.
- 6. FORMATION LEVEL FOLLOW AS PER THE HIGHWAY ALIGNMENT.
 7. THE INVERT LEVEL OF NEW PIPE CULVERT SHALL NOT BE
- 7. THE INVERT LEVEL OF NEW PIPE CULVERT SHALL NOT B HIGHER THAN THE EXISTING INVERT LEVEL
- IF CONSTRUCTION OF HEAD WALL FALLS OUT SIDE ROW, SIDE SLOPE MUST BE ADJUSTED ACCORDINGLY TO ACCOMMODATE THE STRUCTURE INSIDE ROW.
- 9. THE PIPE SHALL BE JOINTED AS PER MORTH SPECIFICATION CLOSE—2906.

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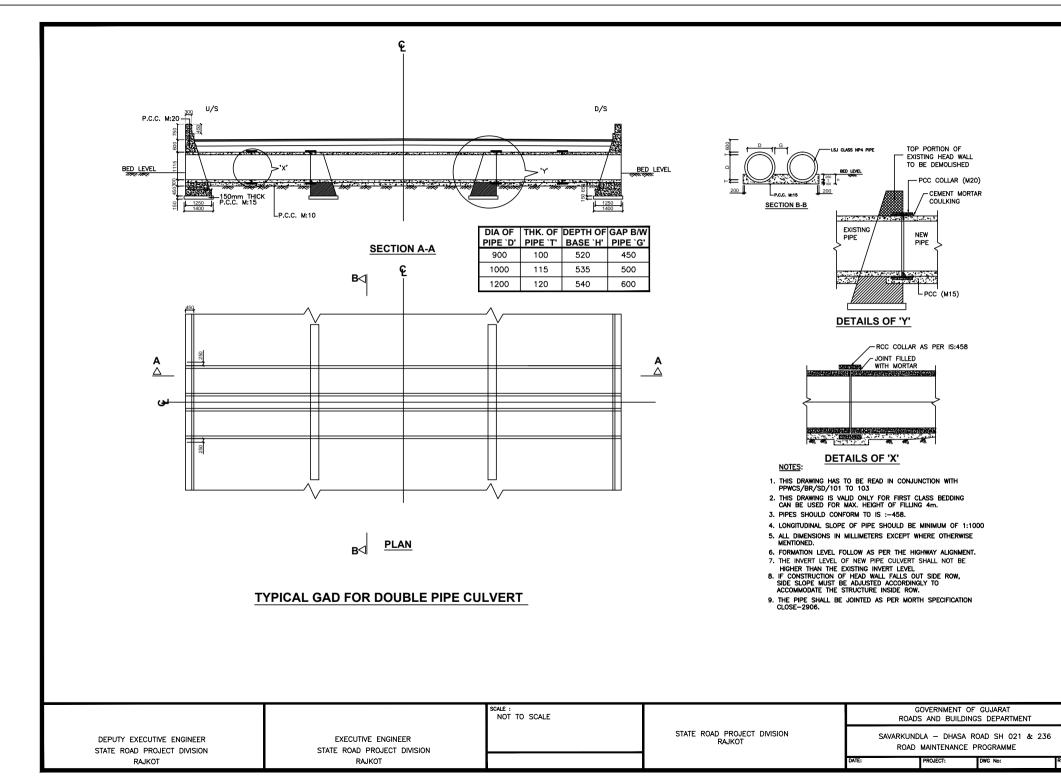
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(A) GENERAL

- The design is according to the following codes :-
- IRC:5 1008
- IRC:6 2010
- IRC:18 2000
- d. IRC:21 2000
- IRC:22 2000
- IRC:78 2000 f.
- g. IRC:83(PART I) -1999
- IRC:83(PART II) -1987
- IRC:SP:33 1989
- Specifications for "Roads & Bridges" (4th revision) by MORTH.
- II. All dimensions are in mm (unless otherwise specified) & chainages are in metre. Only written dimensions shall be followed. No dimensions shall be scaled.
- The following loads have been considered in the design:-
- a. One lane of IRC class 70R or two lanes of IRC class, A. on carriageway, whichever governs,
- b. Footpath load of 5kN/sqm for superstructure having footpaths.
- Wearing coat load of 2kN/sam.
- IV. The designs are applicable for "moderate and "severe" conditions of exposure. In case of "severe" conditions suitable anti-corrosion treatment as approved by the Engineer may be provided to reinforcement bars and exposed concrete surface.

(B) MATERIALS SPECIFICATIONS

Concrete

I. Concrete shall be design mix and have a minimum 28 days characteristic strength As given below on 150mm cubes.

STRUCTURAL ELEMENT	CONCRETE	CHARACTERISTIC S	TRENGTH
Prestressed girders including deck slab Rcc girders including deck slab		40/45 Mpa 35 Mpa	
Piers, Pier cap, Well, Well cap, Abutments, - Wing wall, Rcc railing		35 Mpa	
Pile, Pile cap	_	35 Mpa	
Crash barrier -	-	40 Mpa	

- Ordinary Portland cement conforming to IS:269 or High Strength Ordinary Portland Cement conforming to IS:8112 capable of achieving the required design concrete strength shall only be used.
- To improve workability of concrete and cement grout, admixtures conforming to IS:6925 and IS:9103 could be permitted subject to satisfactory proven use. Admixtures generating hydrogen, nitrogen, chlorides etc. should not be used.
- Cement content in RCC members shall neither be less than 400 kg/cum nor more than 540 ka/cum of concrete.
- Maximum water cement ratio shall be restricted to 0.40.
- The nominal maximum size of aggregate to be used in RCC and PSC work shall be 20mm.
- Minimum clear cover to reinforcement shall be 70mm at surfaces in direct contact With soil and/or water, and 50mm at the remainder, unless otherwise noted.
- 12mm thick plaster in cement mortar 1:3 to be applied on the top surface of the Deck slab before filling Sheathing
- I. Sheathing shall be of "Drossbach" type 75mm ID manufactured from minimum o.4mm thick bright metal strip. It shall be tested as per IRC:18-1985, Appendix 1.
- The joints of all sheathing shall be water tight and conform to provisions contained in appendix-2 of IRC:SP:33-1989. Water
- I. Water to be used in concreting, grouting and curing shall conform to clause 5.1(ii) of IRC:SP:33-1989. Reinforcement
- All reinforcing steel shall be of High Yield Strength TMT-PC Bars (Grade designation Fe 415/Fe 500) conforming to IS: 1786.Mild Steel bars Grade designation S 240 shall confirm to IS: 432 part-1

II. Notation of bar reinforcement shall be as follows:-

Example: -Centre to centre Type of bar as-Bar spacing mentioned above Dia of bar

▼ represent Grade - S: 415 Grade Bar ø represent Grade - S : 230 Grade Bar

Bar spacing indicated on the drawings shall be perpendicular to bars unless indicated in the drawing.

- Steel spacer bars shall be provided between adjacent layers of parallel reinforcement and spaced at not more than 60 x smaller bar dia. The diameter of the spacer bar shall be at least 25mm but not less than the dia of the parallel reinforcements.
- Binding wires should be annealed 16 gauge mild steel wires free from any deleterious matters, dust etc.
- At the locations where reinforcing bars are congested (like girder bulbs), Mechanical splices shall be used instead of over lapping to provide sufficient clearance Between adjacent bars.

Expansion Joints

- The expansion joints must be robust, durable, water tight and replaceable. it must be provided over the full width of superstructure including kerb and footpath following the profile of the same (where relevant), expansion joints shall be obtained only from approved manufacturers and be of proven type. Details of expansion joints must be approved before commencement of construction. Site fabricated expansion joints shall be prohibited.
- II. Expansion joints shall have the following additional essential features:-
- a. For R.C.C.T-beam bridges, It shall cater for a total movement of 20mm with original gap of 40mm between concrete faces.
- b. For PSC Girder Bridges, It shall cater for a total movement of \pm 40mm
- Fabricated steel parts shall be positioned accurately before concreting the portion of deck slab beyond the end faces of webs of box girder.
- IV. Presence of manufacturer's representative at the time of positioning of embedded parts and installation of expansion joints is mandatory.

Weep holes

- I. Weep holes 1000 1.0m c/c staggered to be provided in
- a. Abutment for Major and Minor bridges
- b. Wing wall for Major. Minor bridge and Box structure.
- c. End walls of Box structure
- d. RCC or PCC Earth Retaining Wall.

CONSTRUCTION

Sequence of prestressing

ACTIVITY (After casting of main girders)

14 Stressing of 1st stage cables

21 Casting of deck slab and cross girders

56 Stressing of 2nd stage cables

After 56 Installation of expansions joint and casting/laying of footpath (where applicable), kerb, wearing coat and railing.

Stressing of 1st stage cables can be done earlier on achieving a strength of 35 MPa. Susbsequent activities can also be advanced keeping the same time intervals.

Launching Truss

The design is based on cast-in-situ construction. However launching of girders may be permitted for which the load from leg of launching truss should not exceed the value given in relevant drawing.

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(D) PRESTRESSING

- . The jacking force in each cable is 1543 kN to be imparted at both end simultaneously by using multi- strand jack.
- ll. The following basic properties of Prestress tendons have been considered in the design:—

a. Area of 1 strand

98.7 sqmm

b. Wobble coefficient k

= 0.0046 per metre

c. Friction coefficient u

0.25

d. Modulus of Elasticity of strand

= 1.95*10⁵ MPa = 6mm

e. Anchorage slip

- III. Minimum strength of concrete at the time of tensioning of cables shall not be less than 35 MPa.
- IV. Grouting shall be carried out as per Appendix 2 of IRC:18-2000. After the prestressing operations are completed and strands have been cut by acceptable tools, reusable metal cap with a central hole shall be fixed by four bolts to the guide plate of anchor to prevent leakage during grouting under pressure. Sultable rubber gasket shall be provided at the interface of metal cap and guide. External threaded pipe of 0.D=19mm shall be used as nipple for grouting and shall protude from metal cap. All standard accessories and their details much as vent pipes, fixation details of vent pipe with sheathing, location of vent pipes and methodology of grouting shall be submitted by contractor in consultation with specialised agency for approval of the Engineer.
- V. For future prestressing in case of bridge distress,single 12.7 mm dia. 7-ply class 2 strands as per IS:6006-1983 shall be used. The tensioning force per strand shall be 128.6 KN. Mono strand jacks shall be used for tensioning of strands utilising approved prestressing system only. The externally placed strands shall be protected by polyethelene sheathing and grouted.
- VI. Anchorage recesses to be sealed with epoxy coating and filled with prepackaged non-shrink mortor. End face of girder, at the locations of anchorages, shall be coated with two coats of epoxy.
- VII. Wherever necessary, reinforcement bars may be bent or shifted locally to avoid clashing with prestress tendons and anchorages, holes and recesses.
- VIII. All anchorages plates are to be set at right angles to the tendon.
- IX. Ducts for prestressing steel shall be securely fastened in place to prevent movement Until concrete is placed and hardened. Ducts shall be supported at intervals not exceeding 500mm or as shown on the drawings.
- X. Welding is not permitted within 3000mm of any tendon or tendon duct.
- XI. Prestressing steel shall be accurately located and maintained in position as per drawing, within a maximum tolerance of ± 5mm. The sheathing being supported and fixed at interval not exceeding 0.75m by fixing steel chairs on spacer bars to secondary reinforcement in such a manner that the profiles of the cables is in no way disturbed by heavy vibration and / or by the pressure of wet concrete.
- XII. During concreting and uptill final setting of concrete it shall be ensured that the cable moves freely in the sheath by moving the cables forward and backward.

(E) WORKMANSHIP/DETAILING

- Minimum clear cover to reinforcement shall be 70mm at surfaces in direct contact
 With soil and/or water, and 50mm at the remainder, unless shown otherwise in the drawing.
- II. For ensuring proper cover of concrete to reinforcement bars, the mortar blocks of same grade as of parent concrete shall be provided & should be able to withstand the crushing during construction.
- III. Welding of reinforcement bars shall not be permitted.
- IV. Bending of reinforcement bars to be as per IS:2502-1963.
- V. Minimum lap length shall be kept as 60xd where "d" is the diameter of bar.Lap shall be staggered at such a way that more than 50 bars are lapped at one location.
- VI. Supporting chairs of 12mm dia shall be provided at suitable intervals as per IS:2502.
- VII. Sharp edges of concrete shall be chamfered (25mmx25mm).

- VIII. Formwork details shall be submitted by the contractor for the approval and
- IX. Proper compaction of concrete shall be ensured by use of form and/or needle vibrators. Use of full width screed vibrators for compaction of concrete in deck slab shall be ensured.
- . Shuttering plates shall suitably be stiffened to enable the compaction by form vibrators.
- XI. All setting out dimensions, reduced levels, concrete dimensions & cable profiles to be verified on site before construction commences. Any discrepancy to be brought to the notice of the Engineer immediately.
- XII. The location of jacks for lifting up the superstructure to replace bearing etc. is shown thus This shall be distinctly etched on soffit of superstructure and pier/abutment caps.
- XIII. During jacking operation all jacks placed under one end cross girder shall be operated simultaneously using stress control system so as to ensure that the reaction on both the jacks is eaual at all times.

Construction joints

- Construction joints shall be provided only at locations shown on the drawings. Concreting operation shall be carried out continuously up to the construction joints.
- II. The concrete surface at the joint shall be brushed with a stiff brush after casting while the concrete is still fresh and it has only slightly hardened.
- III. Before new concrete is poured, the surface of old concrete shall be prepared as under :—
- (a) For hardened concrete, the surface shall be thoroughly cleaned to remove debris and laitance and made rough so that 1/4 of the size of aggregate is exposed but without dislodging the aggregate or structurally damaging the concrete.
- (b) For partially hardened concrete, the surface shall be treated by wire brush followed by an air jet. The old surface shall be soaked with water, without leaving puddies, immediately before starting oncreting to prevant absorption of water from new concrete.
- IV. New concrete shall be thoroughly compacted in the region of the joint.

(F) SPECIAL NOTE FOR PRESTRESSING

If the calculated elongation is reached before the calculated gauge pressure is obtained, continue tensioning till the calculated gauge pressure is attained, provided the elongation does not exceed 1.05 times the calculated elongation. If this elongation is achieved before the calculated gauge pressure is attained, stop stressing and inform the Engineer.

If the calculated elongation has not been reached continue tensioning by intervals of 5kg/sqcm till the calculated elongation is attained, provided the gauge pressure does not exceed 1.05 times the calculated gauge pressure.

If the elongation at 1.05 times the calculated gauge pressure is less than 0.95 times the calculated elongation, the following measures must be taken, in succession, to define the cause of this lack of elongation.

- Recalibrate the pressure aguae
- Check the functioning of the jack, pump and leads
- De-tension the cable. Slide it in its duct to check that it is not blocked by mortar which
 has entered through the holes in the sheath. Re-tension the cable, if free.

If the required elongation is not obtained, further finishing operations such as cutting or sealing should not be undertaken without the approval of the Engineer.

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NOTES FOR REPAIR AND REHABILITATION

- 1. OBJECTIVE OF REPAIR TO RCC STRUCTURAL ELEMENTS:-
 - REMOVE CAREFULLY AND WITHOUT DAMAGE TO ADJACENT STRUCTURES TO BE LEFT IN PLACE, ALL SUB-STANDARD MATERIALS.
 - EXPOSE AND PROTECT CORRODED REINFORCEMENT.
 - REPLACE SUB-STANDARD MATERIALS BY NEW SPECIFIED MATERIALS.
- METHOD STATEMENT OF ALL WORKS SHALL BE GIVEN BY THE CONTRACTOR GIVING WORKING DETAILS & DRAWINGS OF METHODOLOGY PREPARED FOR EACH ACTIVITY OF THE REPAIR & REHABILITATION WORKS FOR EACH AFFECTED STRUCTURE.
- REPRESENTATIVE OF MANUFACTURER OF PROPREITARY MATERIALS AND EQUIPMENT TO BE AVAILABLE AT SITE FOR TRAINING CONTRACTOR'S PERSONNEL AND CERTIFYING WORK AS PER THEIR SPECIFICATIONS.
- USE OF SPECIAL PROPRIETARY MATERIALS IS ENVISAGED. THIS IS BASED ON "FOSROC" PRODUCTS. APPROVED EQUIVALENT CAN BE USED.
- 5. USE OF SPECIAL PROPRIETARY MECHANICAL EQUIPMENTS FOR DRILLING, BREAKING, CHIPPING ETC IS ENVISAGED. THE EQUIPMENTS FOR THIS PURPOSE SHALL BE OF SPECIALISED PORTABLE TYPE "HILI"I MANUFACTURE. FOR DEMOLITIONS & BREAKING HILTI EQUIPMENT NO.TE-90 & FOR DRILLING HOLES IN CONCRETE & STEEL HILTI EQUIPMENT NO.TE-95 IS RECOMMENDED. MOBILISATION AND USE OF SUCH EQUIPMENTS SHALL BE CONSIDERED INCIDENTAL TO THE WORKS. NO SEPARATE PAYMENT SHALL MADE ON THIS ACCOUNT. APPROVED EQUIVALENT CAN BE USED.
- 6. PRE-REPAIR TESTS:
- TO UNDERSTAND THE GENERAL EXTENT/DEPTH OF DISTRESS, CONTRACTOR'S REPAIR EXPERT MAY CONDUCT SOME PRE-REPAIR TESTS (AT THE DISCRETION OF ENGINEER) ON SAMPLES OF SLABS AND/OR OTHER PARTS OF STRUCTURE CONFORMING IRC:SP-35AS BELOW:
- CARBONATION TEST: 2% PHENOLPHTHALEIN SOLUTION ON EXPOSED CONCRETE SURFACE
- CHLORIDE TEST: TESTING PIECES OF CONCRETE (FROM GIVEN DEPTH) FOR CHLORIDE CONTENT.
- CORING: FOR ANALYSING IN LABORATORY FOR VARIOUS PROPERTIES INCLUDING CONCRETE STRENGTH.
- PROFOMETER TEST: BEFORE TAKING UP CERTAIN TYPE OF REPAIR ACTIVITIES EG. MAKING HOLES IN REINFORCED CONCRETE ELEMENTS, IT MAY BE ESSENTIAL TO PRE-DETERMINE THE LOCATION OF EXISTING REINFORCEMENT BARS IN THE CONCRETE VICINITY, THIS SHALL BE DONE BY EMPLOYING INSTRUMENT "PROFOMETER-4" AVAILABLE FROM AMIL LTD.,(OR APPROVED EQUIVALENT). THIS INSTRUMENT IS CAPABLE OF DETERMINING THE COVER AND DIA OF THE EXISTING REINFORCEMENT. OTHER METHODS LIKE CHISELING, DRILLING ETC FOR LOCATING EXISTING REINFORCEMENT BARS WOULD NOT BE PERMITTED.
- HALF CELL POTENTIOMETER TEST: FOR DETERMINING EXTENT OF CORROSION OVER EMBEDDED LENGTH OF REBAR FROM EXPOSED END TEST PANELS SIMULATING ACTUAL FIELD CONDITION AS PER 2807.7 OF MOST SPECIFICATION. THUS THE REPAIR TEAM SHOULD COMPLETELY QUANTIFY THE EXTENT OF CORROSION.
- . SCHMIDT HAMMER & OTHER TESTS:

THESE ARE USED TO MEASURE HARDNESS OF CONCRETE SURFACE WHICH CAN BE RELATED TO ITS STRENGTH. THE INSTRUMENT USED IS VERY HANDY. THE PULLOUT METHODS AND PENETRATION RESISTANCE TECHNIQUES ARE ALSO ADDPTED FOR ESTIMATION OF STRENGTH OF CONCRETE AND ASSESSMENT OF ITS OVERALL QUALITY.

· Ultrasonic pulse velocity measurement

The quality of concrete can be assessed by passing through concrete the ultrasonic pulse and measuring the velocity, measured value may be affected by surface texture, moisture content, temperature, specimen size, reinforcement and stress. Co-relations with strength are difficult to make and will be influenced by types and proportions of mix constituent and maturity. Calibration on test cores is essential.

- 7 BOQ IS APPROXIMATE AND SUBJECT TO CHANGE INSPECTIONS ARE TO BE CARRIED OUT JOINTLY & APPROVED BY THE FAICHIFER
- 8 ACCESS FOR INSPECTION & CARRYING OUT ALL REPAIR WORK TO BE ARRANGED BY CONTRACTOR. WORKING PLATFORMS TO BE PROVIDED BY CONTRACTOR SHALL BE SPACIOUS, STABLE AND STRUCTURALLY SOUND. THESE SHALL BE CONSIDERED INCIDENTAL TO THE WORKS. NO SEPARATE PAYMENTS SHALL MADE ON THIS ACCOUNT.
- 9 IN CASE DIRT WALL / ABUTMENT CAP REQUIRE REPAIRS / DISMANTLING OR REPLACEMENT OF BEARING ON ABUTMENT, ONLY PART OF DIRT WALL SHALL BE DISMANTLED AT A TIME TO ENSURE THAT THE APPROACH IS SERVICEABLE AND PERMITS TRAFFIC MOVEMENT ON BRIDGE DIPPING PECTRECATIONS
- 10 IN CASE EXPANSION JOINTS ARE BLOCKED OR DAMAGED THE SAME INCLUDING ANY DEBRIS OF ETC. SHALL BE REMOVED CAREFULLY. THIS SHALL BE CONSIDERED INCIDENTAL TO THE WORK PROVISION OF NEW EXPANSION JOINTS AND NO SEPARATE PAYMENT SHALL BE MADE FOR REMOVAL OF EXISTING JOINT, DEBRIS ETC.
- 11 THE SERVICE LINES, IF ANY SHALL BE CAREFULLY DIVERTED AS DIRECTED BY THE ENGINEER BEFORE DISMANTLING WORK STARTS. THEY SHALL BE RE-INSTATED TO ORIGINAL LOCATION OR AS DIRECTED BY THE ENGINEER IN CHARGE. IT HIS WORK SHALL BE CONSIDERED INCIDENTAL TO ALL DEMOLITION WORKS AND NO SEPARATE PAYMENT SHALL BE MADE ON THIS ACCOUNT.
- 12 STRUCTURAL CONFORE SHALL CONFORM TO "SEVERE" CONDITIONS OF EXPOSURE AS PER MOST SPECIFICATIONS CLAUSE 1703.2. PARTICULAR ATTENTION IS DRAWN TO CLAUSE 1704.1 OF MOST SPECIFICATION INDICATING MINIMUM SLUMPS REQUIREMENTS FOR WHICH SUPERPLASTICISER CONPLAST SP337 OF FOSROC OR APPROVED EQUIVALENT SHALL BE ADDED AS PER MANUFACTURET'S SPECIFICATION.
- 13 ARRANGEMENT OF PASSAGE OF TRAFFIC ALONG A PART OF EXISTING CARRIAGE WAY OR ALONG DIVERSIONS DURING CONSTRUCTION SHALL REMAIN THE RESPONSIBLITY OF THE CONTRACTOR. ADDITIONAL STEEL RESTLES AND PROVISION OF STEEL RAMPS REQUIRED TO FACILITATE PASSAGE OF TRAFFIC ALONG PART OR FULL EXISTING CARRIAGEWAY SHALL BE CONSIDERED INCIDENTAL TO THE WORKS. NO SEPARATE PAYMENT SHALL MADE ON THIS ACCOUNT. METHOD STATEMENT TO BE SUBMITTED BY CONTRACTOR SHALL BE BASED ON THIS PREMISE AND SUBJECT TO APPROVAL OF THE ENGINEER. IN SOME BRIDGES IT MAY BE ESSENTIAL TO DIVERT THE TRAFFIC ALONG A TEMPORARY DIVERSION. THE PAYMENT FOR TEMPORARY TRAFFIC DIVERSION SHEEP IN THE PAYMENT FOR TEMPORARY TRAFFIC DIVERSION WHERE IMPLEMENTED SHALL BE MADE AS PER MODIFIED CLAUSE NO 1126 OF THE SUPPLIMENTRY SPECIFICATIONS OF THE TEMPOR DOCUMENT.
- 14 FLAT JACKS WHEIF USED FOR LIFTING OF SUPERSTRUCTURE OR PORTION THEREOF SHALL BE MANUFACTURE OF OR APPROVED EQUIVALENT.

 HYDRAULIC JACKS USED SHALL BE OF APPROVED MAKE & HAVE THE FACILITIES LIKE HYDRAULIC COUPLING, MECHANICAL LOCKING ETC THESE SHALL INVARIABLY BE
 - STEEL TRESTLE USED FOR LIFTING SUPERSTRUCTURE SHALL BE DESIGNED IN ACCORDANCE WITH IRC:24-1967.

CALIBERATED FREQUENTLY AS DESIRED BY THE ENGINEER.

- JACKS ARE TO BE USED ONLY FOR LIFTING PURPOSES AND SHOULD BE MECHANICALLY LOCKED THEREAFTER. THE LOAD SHOULD BE TRANSFERRED AS SOON AS PRACTICABLE ON TO SEPARATE STEEL / WOODEN PACKINGS AND WEDGES FOR SAFETY.
- 15 FOR BONDING NEW CONCRETE TO OLD CONCRETE, ALL DEFECTIVE OR WEAK CONCRETE SHALL BE REMOVED AS PER SPECIFICATIONS & EQUIPMENTS WITHOUT DAMAGNIG EXISTING REINFORCEMENT OR OTHER EMBEDMENT ITEMS, ALL LOOSE MATERIAL, ARROUND EXISTING EVENOSE CONCRETE & REINFORCEMENT SHALL BE REMOVED BY USING SAND BLASTING OR ANY OTHER APPROVED SCHEME, AFTER CLEANING EXPOSED CONCRETE SURFACE, CONCRETE FURIER SHALL BE APPLIED ON REPEARED CONCRETE SURFACE & REINFORCEMENT SHEEP SHALL BE APPLIED ON REPEARED CONCRETE SURFACE & REINFORCEMENT PRIMER ON EXPOSED REINFORCEMENT AS PER ADDITIONAL SPECIFICATIONS TO FACILITATE PROPER BONDING BETWEEN NEW & OLD CONCRETE.

- 16 FORMWORK SHALL BE LEAK TIGHT TAILOR MADE FOR EACH APPLICATION.
- 17 BEFORE TAKING UP CERTAIN TYPE OF REPAIR ACTIVITIES EG, MAKING HOLES IN REINFORCED CONCRETE ELEMENTS, IT MAY BE ESSENTIAL TO PRE-DETERMINE THE LOCATION OF EXISTING REINFORCEMENT BARS IN THE CONCRETE VICINITY. THIS SHALL BE DONE BY EMPLOYING NON DESTRUCTIVE METHODS WITH THE INSTRUMENT "PROFOMETER-4" AVAILABLE FROM AIMIL LTD., OR APPOVOE EQUIVALENT. THIS INSTRUMENT IS CAPABLE OF DETERMINING THE COVER AND DIA OF THE EXISTING REINFORCEMENT. OTHER METHODS LIKE CHISELLING, DRILLING ETC FOR LOCATING EXISTING REINFORCEMENT BARS WOULD NOT BE PERMITTEN.
- 18 BARS SHOWN THUS Ø ARE MILD STEEL BARS AND SHALL CONFIRM TO IS-432/1982.
- $_{19}$ Bars shown thus $\overline{\varphi}$ are cold worked steel high strength $_{\rm TMT-PC}$ bars and shall confirm to is:1786–1979.
- 20 BENDING AND FIXING OF REINFORCEMENT BARS SHALL BE CARRIED OUT AS PER
- 21 RIVER TRAINING WORKS/PROTECTION WORKS:— LOCATION ALIGNMENT AND SIZE SHALL BE AS DIRECTED BY THE ENGINEER.
- 22 NOT MORE THAN 50% BARS SHALL BE LAPPED AT A LOCATION.
- 23 LAP LENGTH FOR BARS SHALL BE 60xDIA OF BARS (UOS)
- 24 IN CASE OF WIDENING OF SLAB TYPE SUPER STRUCTURE FOR THICKNESS AND REINFORCEMENT DETAIL REFER MOST STANDARD DRAWING. IF THE MOST SPECIFIED SLAB THICKNESS IS LESS THAN THE EXISTING SALB THICKNESS PROVIDE THE EXISTING SLAB THICKNESS IN CONCRETE GRADE M-30 AND REINFORCEMENT DETAIL AS/MOST.
- 25 IN RAPAIR BRIDGES WHERE STEEL RAILING OR ONLY GUARD STONE IS PROVIDED IN EXISTING BRIDGE IN PART OR FULL LENGTH, RCC RAILING (DETAIL DRG. NO:— (PPWCS/BR/DD/O4) TO BE PROVIDED INSTEAD OF STEEL CRASH BARRIER AS SHOWN IN DETAIL DRAWINGS OF REPAIR.
- 26 IN BRIDGES RESTING ON OPEN FOUNDATION AND IDENTIFIED TO BE REHABILITATION / WIDENED SHALL BE REMOVED FROM TOP OF THE FOOTING AFTER REPAIR / WIDENING TOP OF THE FOUNDATION SHALL BE BACK FILLED WITH M-20 PCC TO MATCH THE ORIGINAL BED RIVER LEVEL, AS DIRECTED BY THE DRINKEER.

SCALE:
NOT TO SCALE

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SAVARKUNDLA — DHASA ROAD SH 021 & 236
ROAD MAINTENANCE PROGRAMME

DATE: | PROJECT: | DWG No: | RREV.

DEPUTY EXECUTIVE ENGINEER
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GENERAL STANDARD NOTES

STANDARD NOTES FOR STRUCTURAL STEEL

- ALL BOLTS SPECIFIED IN DRGs. ARE HIGH STRENGTH FRICTION GRIP TYPE (HSFG) OF PROPERTY CLASS 8.8 CONFORMING TO IS: 3757-1985.
- ALL WASHERS SHALL CONFORM TO IS: 6649 1985.
 AND NUTS SHALL CONFORM TO IS: 6623 1985.
- EACH NUT AND BOLT SHALL BE ASSEMBLED WITH AT LEAST ONE WASHER.
- ALL THE HSFG BOLTS SHALL CONFORM TO SP 6 (4)1969, IS 4000 1967
- ALL HOLES IN MEMBERS SHOULD PREFERABLY BE DRILLED BURRS SHOULD BE REMOVED AND THE NOMINAL HOLE DIAMETER SHALL BE 13mm.
- ALL OIL, DIRT, LOOSE RUST, BURRS PAINT APPLIED FINISHES, ANY FORGIN MATERIAL AND ANY OTHER DEFECT ON THE CONTACT SURFACES SHOULD BE REMOVED BY SAND BLASTING. A CLEAN AS ROLLED SURFACE WITH TIGHT MILL SCALE IS ACCEPTABLE.
- PACKING SHALL BE PROVIDED WHEREVER NECESSARY TO ENSURE THAT THE LOAD IS TRANSMITTED EFFECTIVELY. ALL PACKINGS SHALL BE OF STEEL WITH A SURFACE CONDITION SIMILAR TO THAT OF ADJACENT MEMBERS.
- PAINTING OF THE STRUCTURE SHOULD BE CARRIED OUT AT AN EARLY STAGE AFTER TIGHTENING AND INSPECTION OF THE JOINTS TO PREVENT RUSTING IN CORROSIVE ATMOSPHERE. PAINTING SHALL BE CARRIED OUT AS PER MOST STANDARDS AND AS DIRECTED BY ENGINEER.
- METHODS OF TIGHTENING OF ALL HSFG BOLTS SHALL BE ONE OF THE FOLLOWING.
 - (i) CALIBRATED WRENCH TIGHTENING.
 - (ii) TURN OF NUT OR PART TURN TIGHTENING.
 - (iii) TIGHTENING BY USE OF DIRECT TENSION INDICATOR.
- "REUSE OF ANY HSFG BOLT AND NUT" SHOULD NOT BE DONE ONCE IT IS FULLY TIGHTENED.
- IT IS ADVISED TO MARK ALL TIGHTENED CONNECTIONS SO THAT THERE IS NO CONFUSION BETWEEN TIGHTENED BOLT AND YET TO BE TIGHTENED BOLTS.

SCALE:
NOT TO SCALE

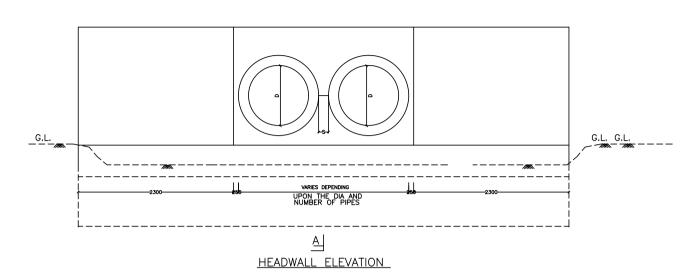
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RAJKOT

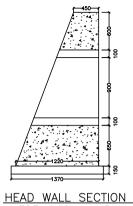
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ROAD MAINTENANCE PROGRAMME

PROJECT: DWG No: REV.

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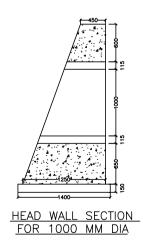






FOR 900 MM DIA

HEAD WALL SECTION FOR 1200 MM DIA



LEGEND:

D = DIAMETER OF PIPE.

 $s = SPACING OF PIPES = 1/2 \times D$

NOTE:

THE SIMILAR HEADWALL DETAILS IS APPLICABLE TO SINGLE AND MULTIPLE PIPE CULVERTS.

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EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION **RAJKOT**

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GOVERNMENT OF GUJARAT ROADS AND BUILDINGS DEPARTMENT

SAVARKUNDLA - DHASA ROAD SH 021 & 236 ROAD MAINTENANCE PROGRAMME

	SAVARKUNDLA - DHASA ROAD SH - 236 & SH - 021	
	INDEX	
Sr. No.	DRAWING TITLE	DRAWING NO.
1	TYPICAL CROSS SECTION	
2	ALIGNMENT AND L - SECTION	
3	SCHEDULE (CRASH BARRIER, PEDESTRIAN CROSSING & RUMBLE STRIP)	
ROAD	 MAINTENANCE PROGRAMME	
4	DIVERSION OF TRAFFIC ON HALF OF THE ROAD WIDTH (SAY LEFT)	
5	EXCAVATION AND EXTENSION OF SHOULDER & FILLING OG G.S.B. MATERIAL ON SHOULDER	
6	SHALLOW POT HALE REPAIRS	
7	PREPARATORY / REPAIR WORK LESS THAN 75MM IN DEPTH	
8	CROSS SECTION OF DEPRESSION MORE THAN 75MM DEPTH FOR FULL DEPTH REPAIR.	
9	TREATMENT FOR FULL DEPTH REPAIR	
10	SEALING OF WIDE CRACKS > 3MM	
11	SEALING OF FINE / NARROW CRACKS < 3MM	
12	CENTER LINE MARKING ON STRAIGHT ALIGNMENT & ON HORIZONTAL CURVE	
13	SPEED BUMP / RUMBLE STRIPS	
14	PROFILE CORRECTIVE COURSE & BITUMINOUS TREATMENT	
15	DETAIL OF GUARD RAIL (CRASH BARRIER)	
16	DETAILS OF ROAD SIGN - 1	
17	DETAILS OF ROAD SIGN - 2	
18	DETAILS OF LANE MARKING & NIGHT TIME SAFETY (POSITION OF ROAD STUDS)	
19	RAISED PEDESTRIAN CROSSING WITH ROAD STUDS	
20	TRAFFIC CONTROL AND SAFETY DEVICES	
21	DETAILS OF CONCRETE DELINEATOR LAY OUT	
22	R C C GUARD RAILS FOR BRIDGE	
23	R C C GUARD RAILS FOR BRIDGE	
24	R C C GUARD RAILS WITH KERB FOR BRIDGE	
25	R C C GUARD RAILS FOR BRIDGE REINFORCEMENT SCHEDULE	
26	DETAILS OF R. C. C. JACKETING	
27	DETAILS OF C. C. CRACK REPAIR	
28	DETAILS OF R. C. C. CRACK REPAIR	
29	TYPICAL GAD FOR PIPE CULVERT (SINGLE PIPE)	
30	TYPICAL GAD FOR PIPE CULVERT (DOUBLE PIPE)	
31	TYPICAL GAD FOR PIPE CULVERT (PIPE ARRANGEMENT & HEAD WALL)	
32	GENERAL NOTES FOR CONCRETE WORKS	
33	GENERAL NOTES FOR CONCRETE WORKS	
34	GENERAL NOTES FOR CONCRETE WORKS	
35	GENERAL STANDARD NOTES	
36	TYPICAL DETAILS OF TREE GUARD	

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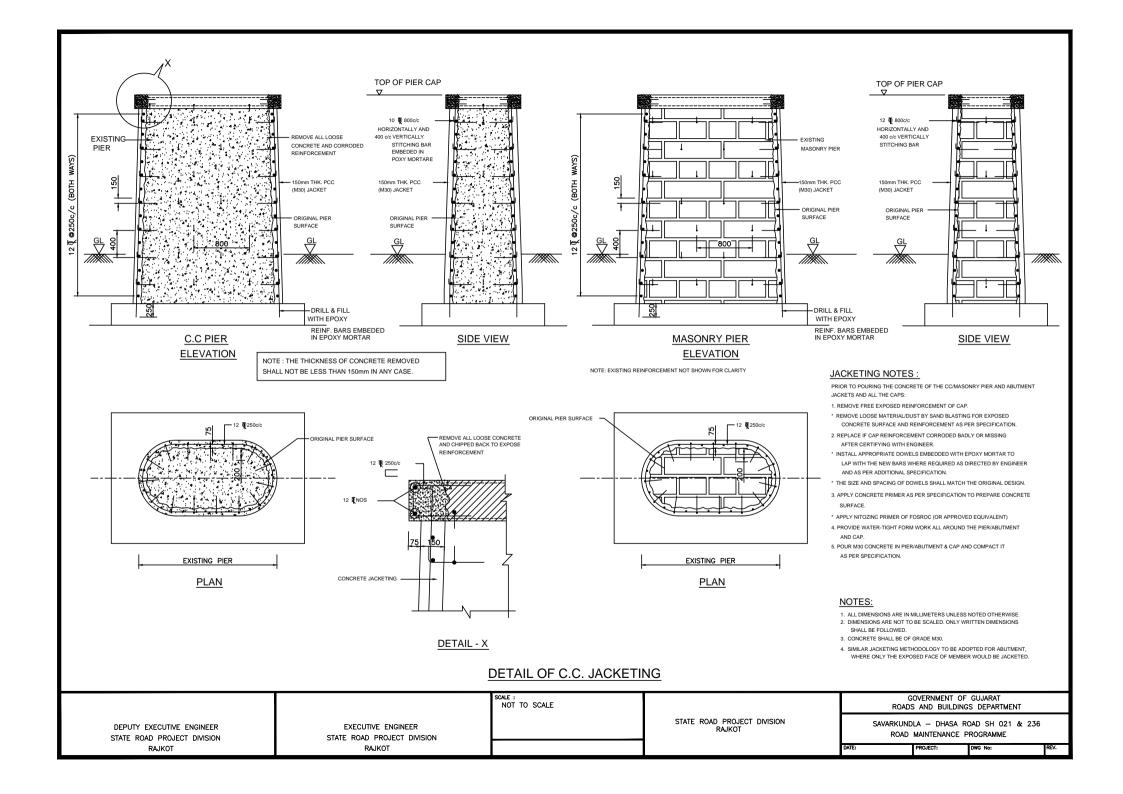
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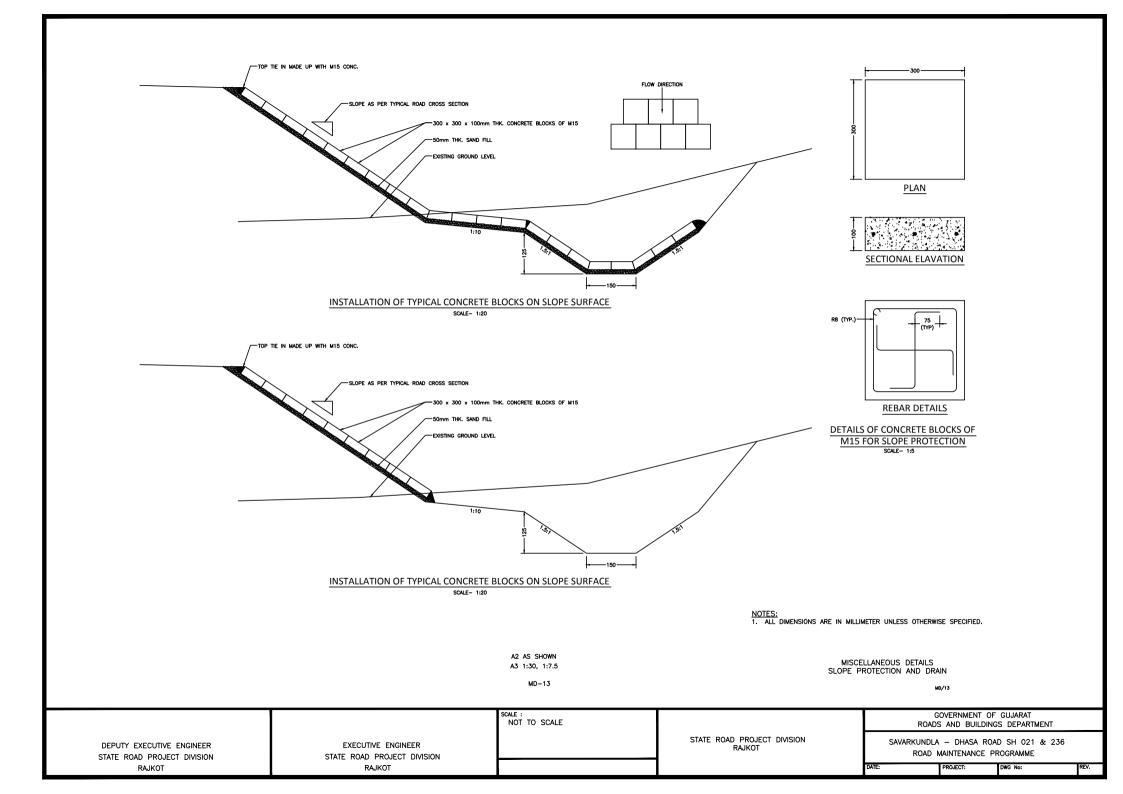
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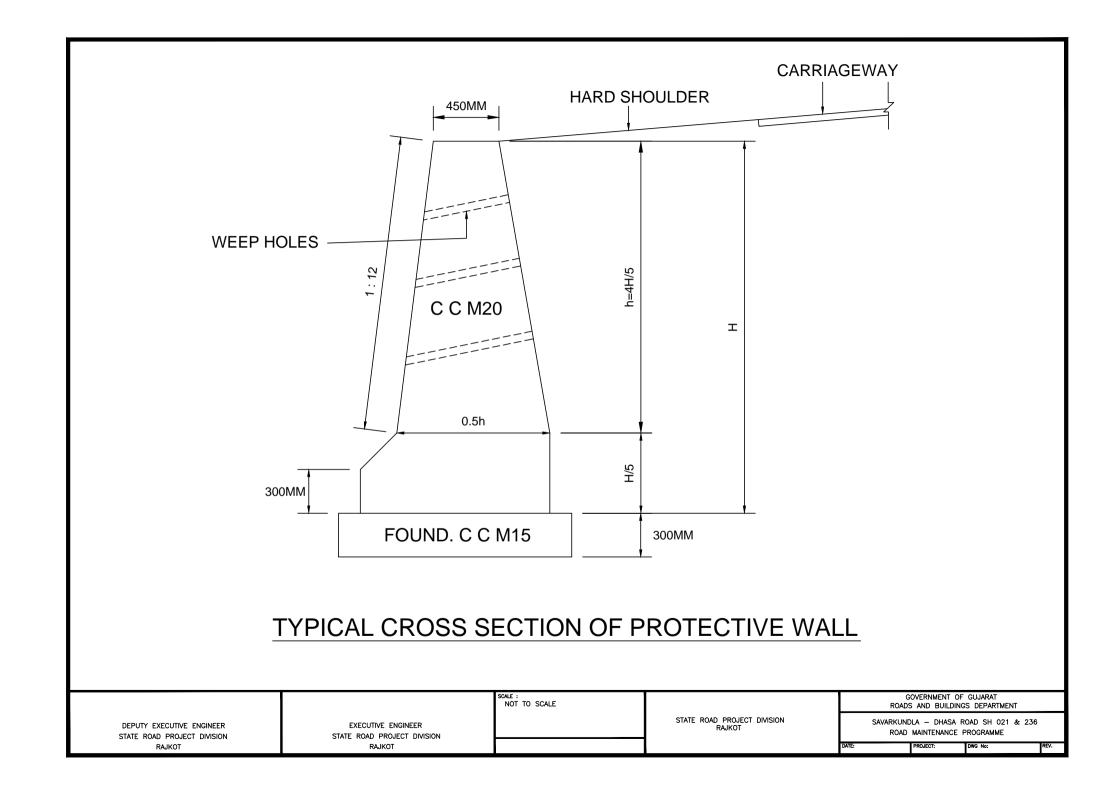
GOVERNMENT OF GUJARAT
ROADS AND BUILDINGS DEPARTMENT

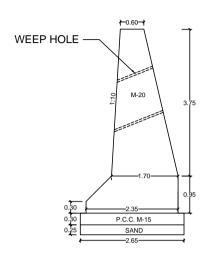
SAVARKUNDLA — DHASA ROAD SH 021 & 236
ROAD MAINTENANCE PROGRAMME

PROJECT: DWG No: REV.

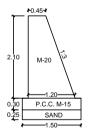




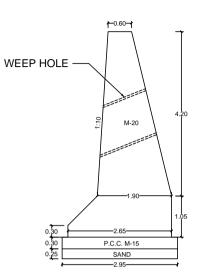




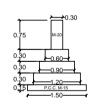
TYPICAL SECTION
OF R.WALL FOR NAVALI BRIDGE
SH-236, CH-10+475



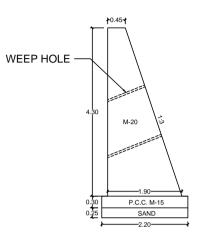
TYPICAL SECTION
OF PROCTECTIVE WALL IN SPILL SECTION
BETWEEN CH-13+100 TO 14+500, SH-236



TYPICAL SECTION
OF R.WALL FOR GAGADIO CH-27+490 &
KHARI BRIDGE CH-32+117 SH-236

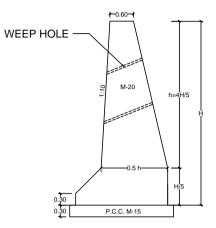


TYPICAL SECTION
OF APROAN CURTAIN WALL CH-28+690,
43+018 SH-236 & CH-12+300 SH-021



TYPICAL SECTION
OF PROCTECTIVE WALL AT
FOLLOWING CHAINAGE SH-236

CHAINAGE	LHS	RHS
40+800 TO 41+000	200	
41+000 TO 41+300	300	300
41+300 TO 41+700	400	
43+750 TO 43+800	50	
44+420 TO 45+350	930	
45+250 TO 45+550	200	200
45+550 TO 46+350	800	
46+350 TO 46+500	150	150
46+500 TO 46+775	275	-



TYPICAL SECTION OF R.WALL

TYPICAL SECTION OF RETAINING WALL, PROTECTIVE WALL & CURTAIN WALL

SOLE:
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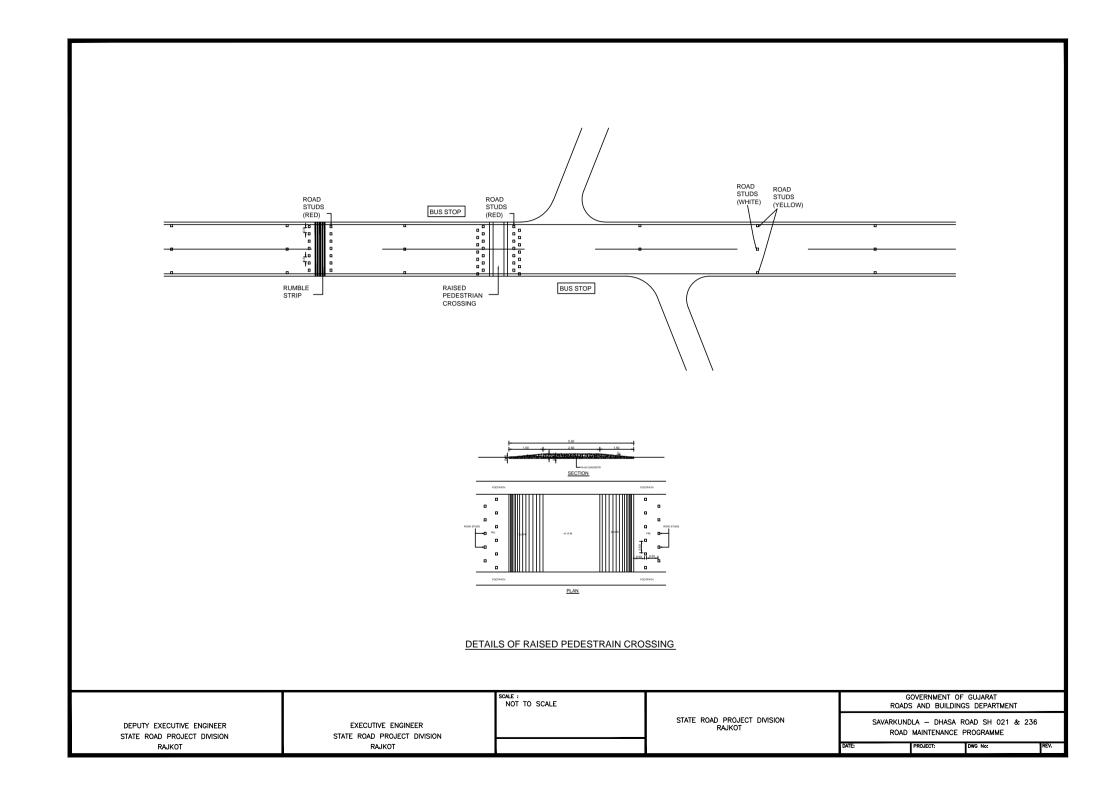
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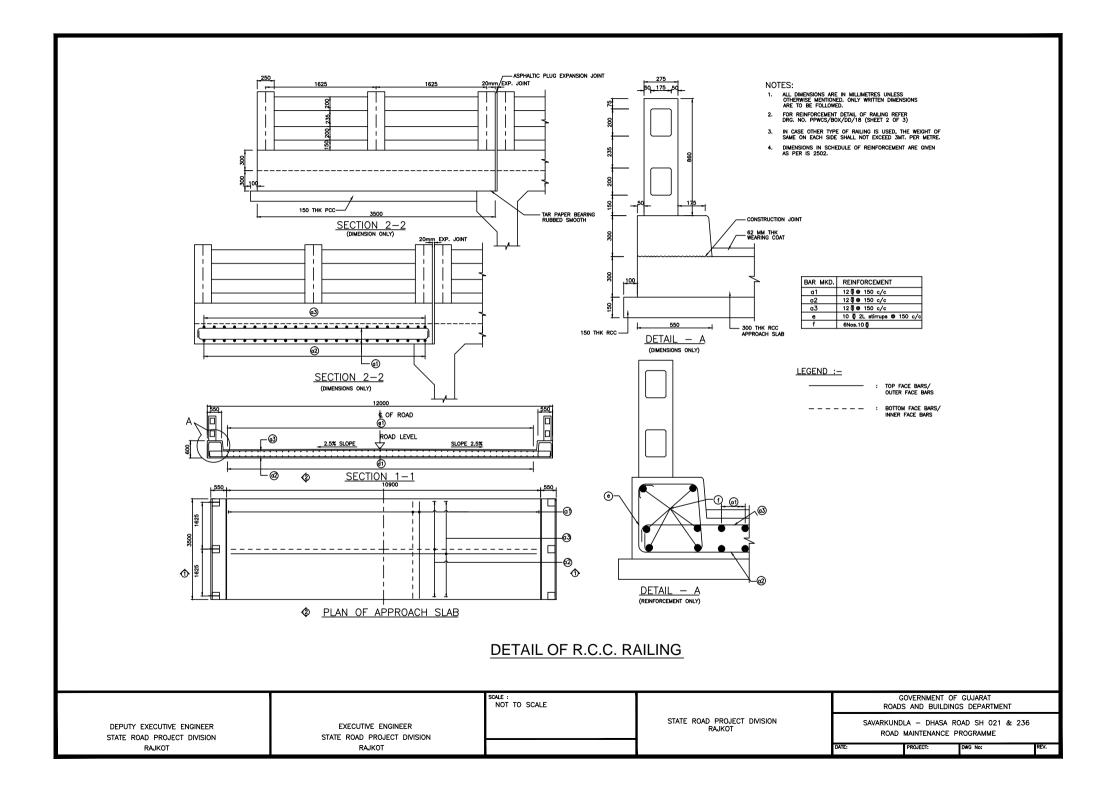
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STATE R





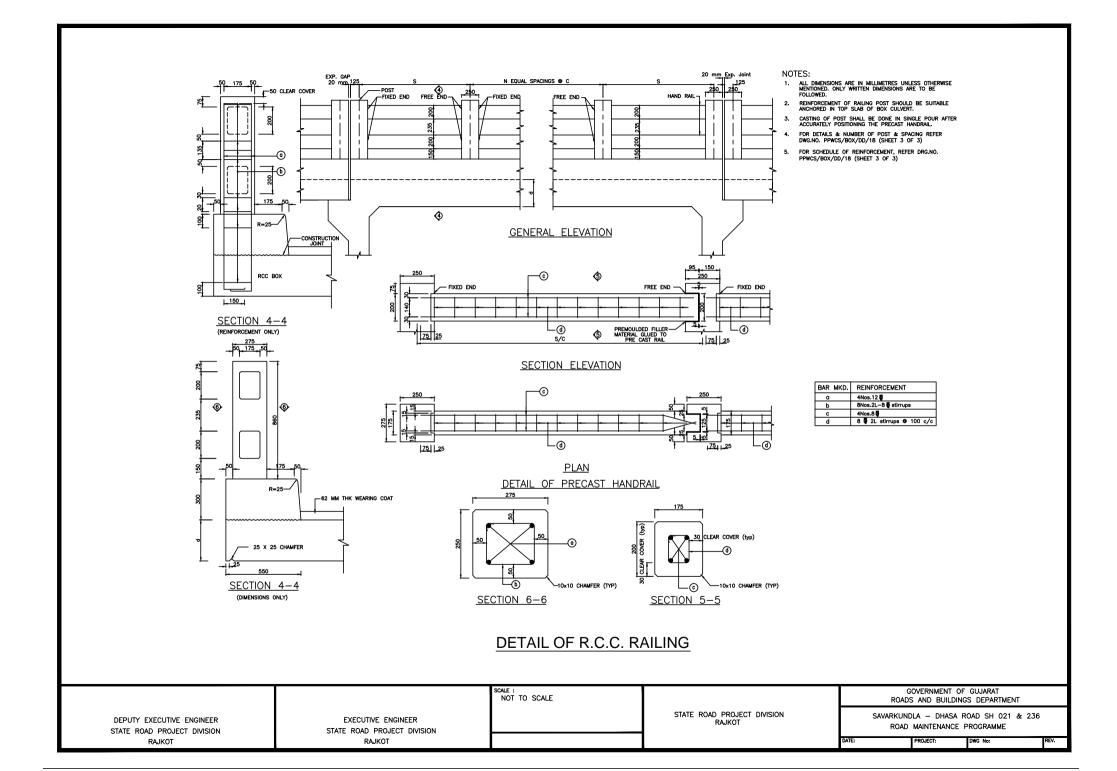


TABLE: BAR BENDING SCHEDULE FOR RAILINGS AND POSTS

	-	RAILI	NG POS	т —										END R	WILING			INTERM	EDIATE RAIL	JING		END F	RAILING				EDIATE RAI	LING
보호	WIDTH	IVAL	10 100	'		R MKD @			8 5 F	MKD (b)				MKD© 4 NOS.	200			MKD@	200_			WKD @ RINGS				NKD @ RINGS		
ĭ≨	ا≩ږا					R POST	150			INUS 5. PER PO	sī Ļ"J"	•	PER				PER				9 10		i	•	i e io		;	•
BOX - CELL DESIGNATION	H TOTAL	s	c .	N .	- LENGTH	NOS.	TOTAL LENGTH Ixn	WEIGHT w1	LENGTH	NOS.	TOTAL LENGTH Isn	WEIGHT #1	LENGTH	NOS.	TOTAL LENGTH Isn	WEIGHT w1	LENGTH	NOS. -2xmp	TOTAL LENGTH Ixn	WEIGHT W1	LENGTH	NOS. -2xmp	TOTAL LENGTH Ixn	WEIGHT W1	LENGTH	NOS.	TOTAL	WEIGHT #1
N/ab/EC	(mm)	(mm)	(mm)	(mm)	(mm)	è	(m)	(kg)	(mm)	ë	(m)	(kg)	(mm)	è	(m)	(kg)	(mm)	~ !	(m)	(kg)	(mm)	è	(m)	(kg)	(mm)	- !	(m)	(kg)
1/22/0	2600	1175	0	۰	1593.80	24	38.25	33.96	677.33	48	32.51	12.83	1100.00	32	35.20	13.89	0.00	0	0.00	0.00	537.33	80	42.99	16.96	0.00	0	0.00	0.00
1/23/0	2700	1225	0	•	1613.80	24	38.73	34.39	677.33	48	32.51	12.83	1150.00	32	36.80	14.52	0.00	۰	0.00	0.00	537.33	80	42.99	16.96	0.00	0	0.00	0.00
1/33/0	3840	1500	590	1	1663.80	32	53.24	47.27	677.33	64	43.35	17.10	1425.00	32	45.60	17.99	515.00	16	8.24	3.25	537.33	104	55.88	22.05	537.33	16	8.60	3.39
1/34/0	3920	1500	670	1	1693.80	32	54.20	48.12	677.33	64	43.35	17.10	1425.00	32	45.60	17.99	595.00	16	9.52	3.76	537.33	104	55.88	22.05	537.33	20	10.75	4.24
1/43/0	5000	1500	875	2	1693.80	40	67.75	60.15	677.33	80	54.19	21.38	1425.00	32	45.60	17.99	800.00	32	25.60	10.10	537.33	104	55.88	22.05	537.33	56	30.09	11.87
1/44/0	5100	1500	925	2	1723.80	40	68.95	61.22	677.33	80	54.19	21.38	1425.00	32	45.60	17.99	850.00	32	27.20	10.73	537.33	104	55.88	22.05	537.33	56	30.09	11.87
1/45/0	5260	1500	1005	2	1763.80	40	70.55	62.64	677.33	80	54.19	21.38	1425.00	32	45.60	17.99	930.00	32	29.76	11.74	537.33	104	55.88	22.05	537.33	64	34.39	13.57
1/53/0	6140	1500	1445	2	1743.80	40	69.75	61.93	677.33	80	54.19	21.38	1425.00	32	45.60	17.99	1370.00	32	43.84	17.30	537.33	104	55.88	22.05	537.33	96	51.58	20.35
1/54/0	6240	1500	1495	2	1783.80	40	70.55	62.64	677.33	80	54.19	21.38	1425.00	32	45.60	17.99	1420.00	32	45.44	17.93	537.33	104	55.88	22.05	537.33	104	55.88	22.05
1/55/0	6400	1500	1050	3	1823.80	48	87.54	77.72	677.33	96	65.02	25.66	1425.00	32	45.60	17.99	975.00	48	46.80	18.47	537.33	104	55.88	22.05	537.33	96	51.58	20.35
1/63/0	7440	1500	1397	3	1923.80	48	92.34	81.98	677.33	96	65.02	25.66	1425.00	32	45.60	17.99	1321.67	48	63.44	25.03	537.33	104	55.88	22.05	537.33	144	77.38	30.53
1/64/0	7500	1500	1417	3	1923.80	48	92.34	81.98	677.33	96	65.02	25.66	1425.00	32	45.60	17.99	1341.67	48	64.40	25.41	537.33	104	55.88	22.05	537.33	144	77.38	30.53
1/65/0	7500	1500	1417	3	1923.80	48	92.34	81.98	677.33	96	65.02	25.66	1425.00	32	45.60	17.99	1341.67	48	64.40	25.41	537.33	104	55.88	22.05	537.33	144	77.38	30.53
1/66/0	7700	1500	1483	3	1943.80	48	93.30	82.84	677.33	96	65.02	25.66	1425.00	32	45.60	17.99	1408.33	48	67.60	26.67	537.33	104	55.88	22.05	537.33	156	83.82	33.08
1/75/0	8700	1500	1363	4	1993.80	56	111.65	99.13	677.33	112	75.86	29.93	1425.00	32	45.60	17.99	1287.50	64	82.40	32.51	537.33	104	55.88	22.05	537.33	192	103.17	40.71
1/76/0	8900	1500	1413	4	2023.80	56	113.33	100.62	677.33	112	75.86	29.93	1425.00	32	45.60	17.99	1337.50	64	85.60	33.78	537.33	104	55.88	22.05	537.33	192	103.17	40.71
1/77/0	8900	1500	1413	4	2023.80	56	113.33	100.62	677.33	112	75.86	29.93	1425.00	32	45.60	17.99	1337.50	64	85.60	33.78	537.33	104	55.88	22.05	537.33	192	103.17	40.71
1/85/0	9900	1500	1330	5	2043.80	64	130.80	116.13	677.33	128	86.70	34.21	1425.00	32	45.60	17.99	1255.00	80	100.40	39.62	537.33	104	55.88	22.05	537.33	220	118.21	46.64
1/86/0	10000	1500	1350	5	2063.80	64	132.08	117.27	677.33	128	86.70	34.21	1425.00	32	45.60	17.99	1275.00	8	102.00	40.25	537.33	104	55.88	22.05	537.33	220	118.21	46.64
1/87/0	10400	1500	1430	5	2093.80	64	134.00	118.97	677.33	128	86.70	34.21	1425.00	32	45.60	17.99	1355.00	80	106.40	42.77	537.33	104	55.88	22.05	537.33	240	128.96	50.89
2/22/0	5040	1500	895	2	1643.80	40	65.75	58.38	677.33	80	54.19	21.38	1425.00	32	45.60	17.99	820.00	32	26.24	10.35	537.33	104	55.88	22.05	537.33	56	30.09	11.87
2/23/0	5100	1500	925	2	1643.80	40	65.75	58.38	677.33	80	54.19	21.38	1425.00	32	45.60	17.99	850.00	32	27.20	10.73	537.33	104	55.88	22.05	537.33	56	30.09	11,87
2/32/0	7100	1500	1283	3	1693.80	48	81.30	72.18	677.33	96	65.02	25.66	1425.00	32	45.60	17.99	1208.33	48	58.00	22.89	537.33	104	55.88	22.05	537.33	132	70.93	27.99
2/33/0	7190	1500	1313	3	1713.80	48	82.26	73.03	677.33	96	65.02	25.66	1425.00	32	45.60	17.99	1238.33	49	59.44	23.45	537.33	104	55.88	22.05	537.33	132	70.93	27.99
3/22/0	7340	1500	1363	3	1613.80	48	77.46	68.77	677.33	96	65.02	25.66	1425.00	32	45.60	17.99	1288.33	48	61.84	24.40	537.33	104	55.88	22.05	537.33	144	77.38	30.53
3/33/0	10740	1500	1498	5	1693.80	64	108.40	96.24	677.33	128	86.70	34.21	1425.00	32	45.60	17.99	1423.00	80	113.84	44.92	537.33	104	55.88	22.05	537.33	260	139.71	55.13
App Slab	3500	1625	0	0	1543.80	48	74.10	65.79	677.33	96	65.02	25.66	1550.00	64	99.20	39.14	0.00	0	0.00	0.00	537.33	224	120.36	47.49	0.00	0	0.00	0.00

BOX - CELL DESIGNATION	10 🧗	MKD (6) Rings (0 C/C in	KERB 7		BAR 10 d PER	TOTAL			
N/ab/EC	ENGTH	NOS. n=2xnp	TOTAL 3 LENGTH	(S) WEIGHT	(all LENGTH	NOS. n=2xnp	TOTAL 3 LENGTH Ism	S WEIGHT	⊬≱ (kg)
1/22/0	2040.58	34	69.38	42.78	2665.66	12	31.99	19.72	140.1
1/23/0	2082.31	36	74.96	46.22	2765.66	12	33.19	20.46	145.4
1/33/0	2186.65	50	109.33	67.41	3905.66	12	46.87	28.90	207.4
1/34/0	2249.26	52	116.96	72.11	3985.66	12	47.83	29.49	214.9
1/43/0	2249.26	66	148.45	91.53	5065.66	12	60.79	37.48	272.6
1/44/0	2311.86	68	157.21	96.92	5165.66	12	61.99	38.22	280.4
1/45/0	2395.33	70	167.67	103.38	5325.66	12	63.91	39.40	292.2
1/53/0	2353.60	82	192.99	118.99	6205.66	12	74.47	45.91	325.9
1/54/0	2395.33	82	196.42	121.10	6305.66	12	75.67	46.65	331.8
1/55/0	2520.54	84	211.73	130.54	6465.66	12	77.59	47.84	360.6
1/63/0	2729.22	98	267.46	164.90	7505,66	12	90.07	55.53	423.7
1/64/0	2729.22	100	272.92	168.27	7565.66	12	90.79	55.97	427.9
1/65/0	2729.22	100	272.92	168.27	7565.66	12	90.79	55.97	427.9
1/66/0	2770.96	102	282.64	174.26	7765.66	12	93.19	57.45	440.0
1/75/0	2875.30	116	333.53	205.64	8765.66	12	105.19	64.85	512.8
1/76/0	2937.90	118	346.67	213.74	8965.66	12	107.59	66.33	525.1
1/77/0	2937.90	118	346.67	213.74	8965.66	12	107.59	66.33	525.1
1/85/0	2979.64	132	393.31	242.49	9965.66	12	119.59	73.73	592.9
1/88/0	3021.37	132	398.82	245.89	10065.66	12	120.79	74.47	598.8
1/87/0	3083.98	138	425.59	262.39	10465.66	12	125.59	77.43	626.7
2/22/0	2144.92	66	141.56	87.28	5105.66	12	61.27	37.77	267.1
2/23/0	2144.92	68	145.85	89.92	5165.66	12	61.99	38.22	270.5
2/32/0	2249.26	94	211.43	130.35	7165.66	12	85.99	53.01	372.1
2/33/0	2290.99	96	219.94	135.60	7255.66	12	87.07	53.68	379.5
3/22/0	2082.31	98	204.07	125.82	7405.66	12	88.87	54.79	370.0
3/33/0	2249.26	142	319.39	196.92	10805.66	12	129.67	79.95	547.4
App Slab	1936.24	92	178.13	109.83	3565.66	24	85.58	52.76	340.7

TABLE: BAR BENDING SCHEDULE FOR APPROACH SLAB

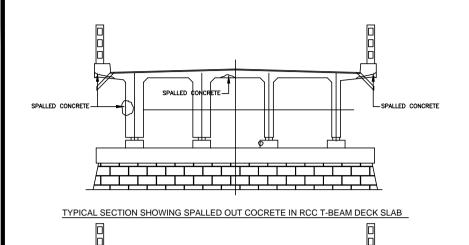
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HDWGH -	NOS.	3 LENGTH	(E) WEIGHT	Э сыстн	NOS. n=2xnp	TOTAL 3 LENGTH bn	(s) WEIGHT	हैं फ़िलम	NOS.	3 LENGTH	(S) WEIGHT	S TOTAL W
3758.8	292	1097.57	974.44	11159	46	513.30	455.72	11162.8	46	513.49	455.88	1886.04

NOTES:-

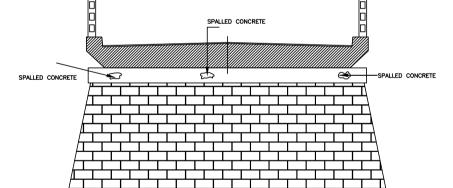
- 1. FOR GENERAL NOTES REFER DWG. NO. PPWCS/GEN/101
- 2. THIS DRAWING SHALL BE READ IN CONJUNCTION TO DWG. NO. PPWCS/BOX/DD/18 (SHEET 1&2 OF 3)

DETAIL OF R.C.C. RAILING

DEPUTY EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION		SCALE : NOT TO SCALE		GOVERNMENT OF GUJARAT ROADS AND BUILDINGS DEPARTMENT						
	EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION		STATE ROAD PROJECT DIVISION RAJKOT	SAVARKUNDLA - DHASA ROAD SH 021 & 236 ROAD MAINTENANCE PROGRAMME						
RAJKOT	RAJKOT			DATE:	PROJECT:	DWG No:	REV.			

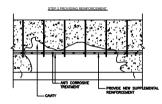


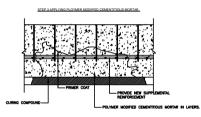




TYPICAL SECTION SHOWING OUT CONC. IN RCC PIER CAP

300000 EXTENT OF CORROSION REMOVAL EXTENT OF CONCRETE REMOVAL EXTENT OF CONCRETE REMOVAL





Step-1

- 1. Demarcate the damaged area
- 2. Remove plaster cover or coating. If any
- 3. Remove all loose concrete and expose rusted reinforcement
- Identify extent of corrosion of reinforcement and remove concrete 300mm beyond rusted length and remove plaster 600mm beyond rusted length.
- Cut edges of concrete base neat and square and in rectilinear pattern.
- 6. Hack back to sound concrete and roughen exposed concrete.
- 7. Clean spalled concrete with filtered compressed air.

Step-2

- 1. Cut and remove any severely corroded and detached reinforcement.
- Clean remaining reinforcement with the help of specified anticorrosive treatment.
- Provide and anchor new supplemental reinforcement and/or wire mesh as required.

Step-3

- Primer slurry coat shall be applied with the help of stiff nylon bristle brush on spalled out surface.
- Before the primer coat is fully cured. Polymer modified cementations mortar shall be applied by means of trowels and floats.
- 3. The interval between the application of primer coat and pmc mortar shall be 15 to 30 minutes depending upon the ambiet temperature.
- The total thickness of PMC mortor shall be applied in multiple layers of thickness 25mm or manufacture's recommendations by trowel.
- 5. Repaired surface shall be matched with original existing surface and curing be carried out by curing compound.

DETAIL OF C.C. CRACK REPAIR

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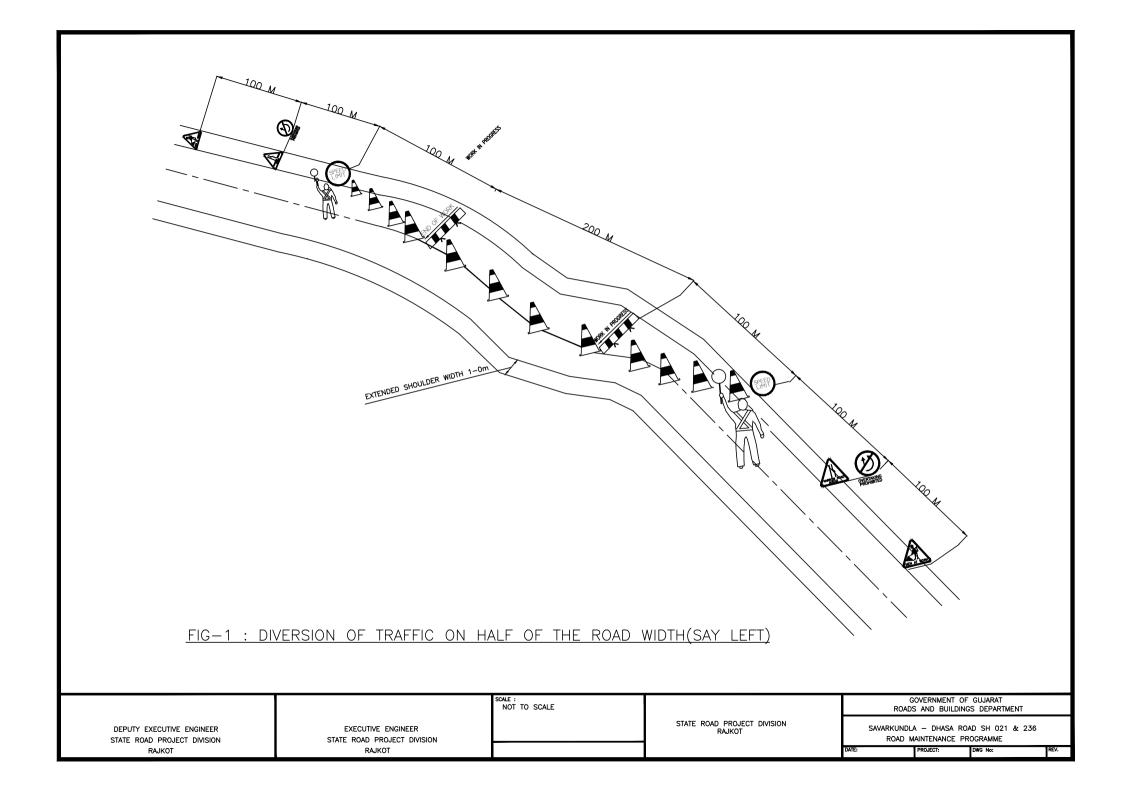
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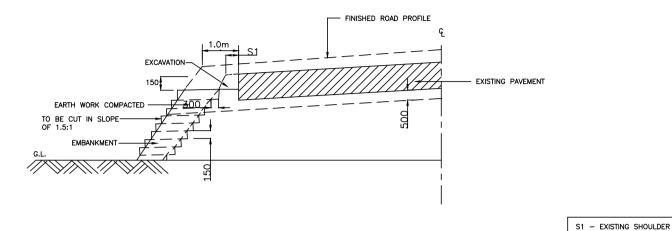


FIG-II: EXCAVATION AND EXTENSION OF SHOULDER

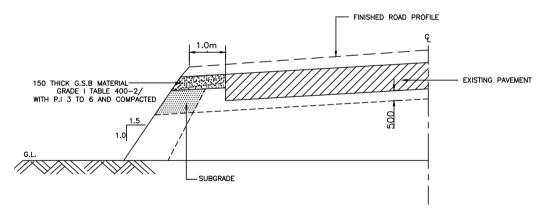


FIG-2: FILLING OF G.S.B MATERIAL ON SHOULDER

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ROAD MAINTENANCE PROGRAMME

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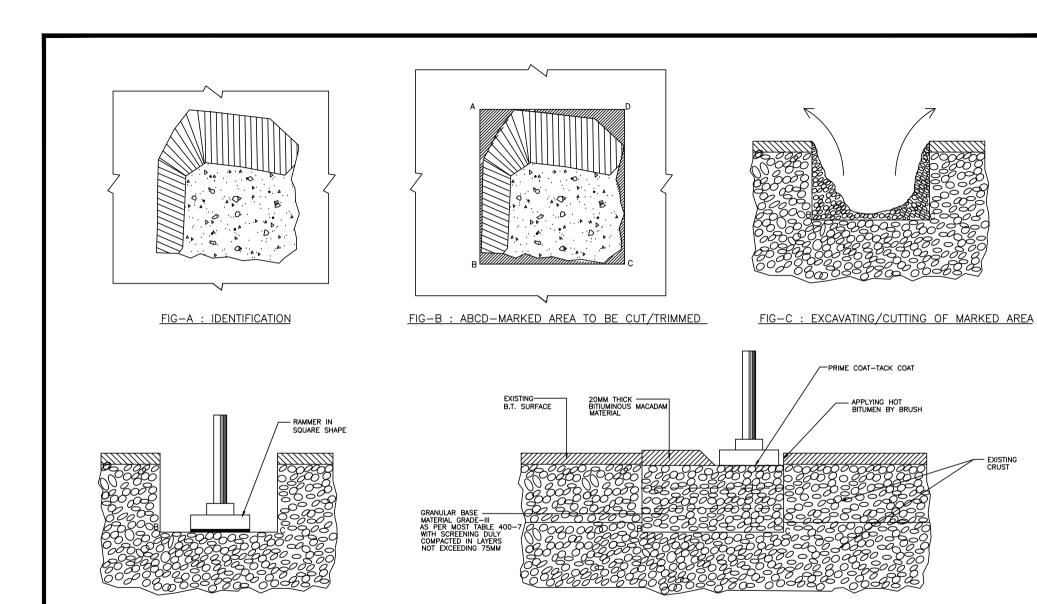


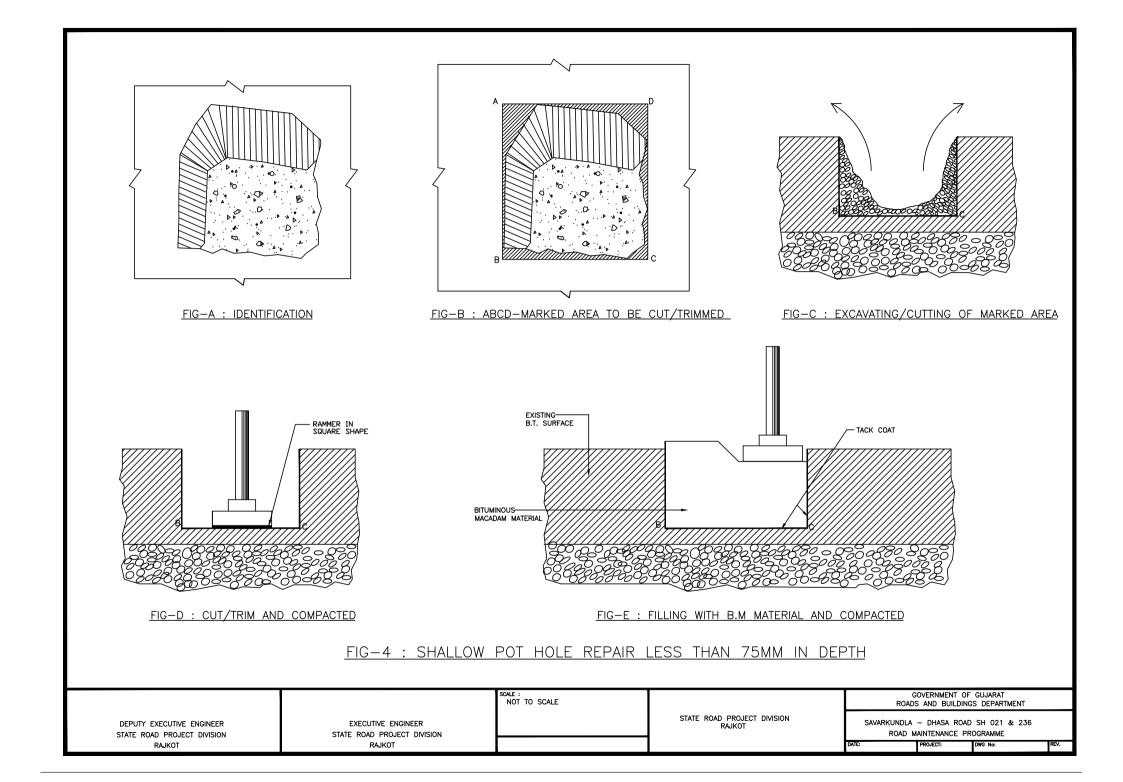
FIG-D : CUT/TRIM AND COMPACTED

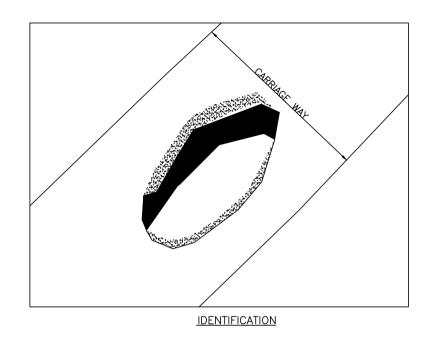
FIG-E: BACKFILLING AND COMPACTING THE HOLE

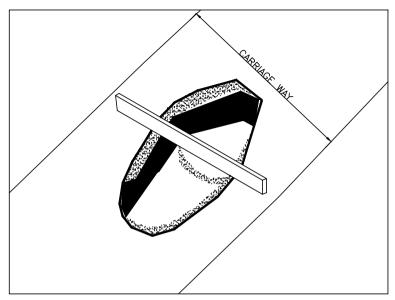
EXISTING CRUST

FIG-3: DEEP POT HOLE/DEPRESSION REPAIR (AREA LESS THAN 1 SQ-M AND DEPTH > 75MM)

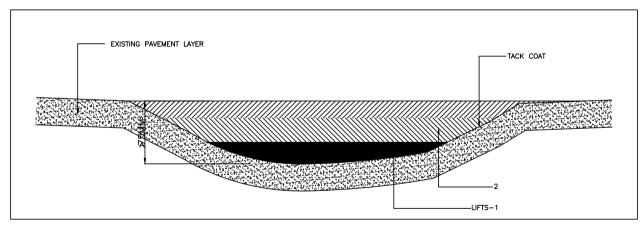
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DEPUTY EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION RAJKOT	EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION RAJKOT		STATE ROAD PROJECT DIVISION RAJKOT	ROAD M	AINTENANCE PR	AD SH 021 & 236 ROGRAMME	REV.			







MARKING OUT AREA



FILLING WITH B.M MATERIAL AND COMPACTED

FIG-5: PREPARATORY/REPAIR WORK FOR DEPRESSION LESS THAN 75MM

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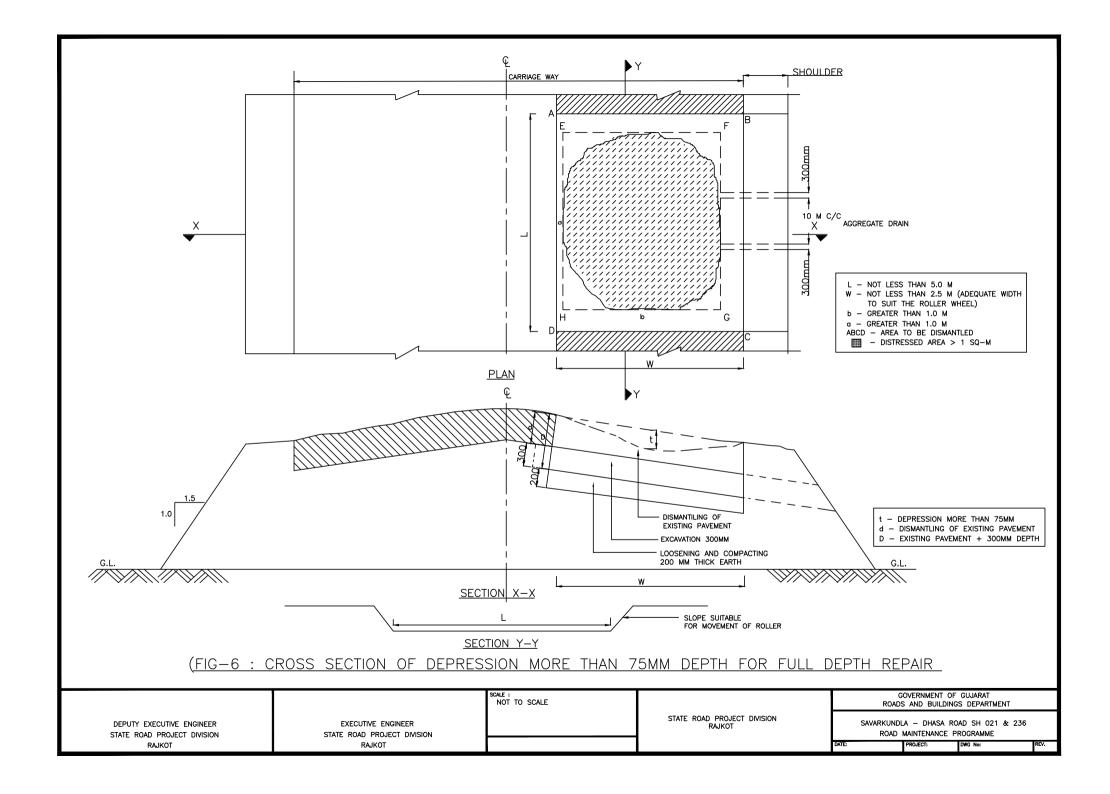
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STATE ROAD PROJECT DIVISION
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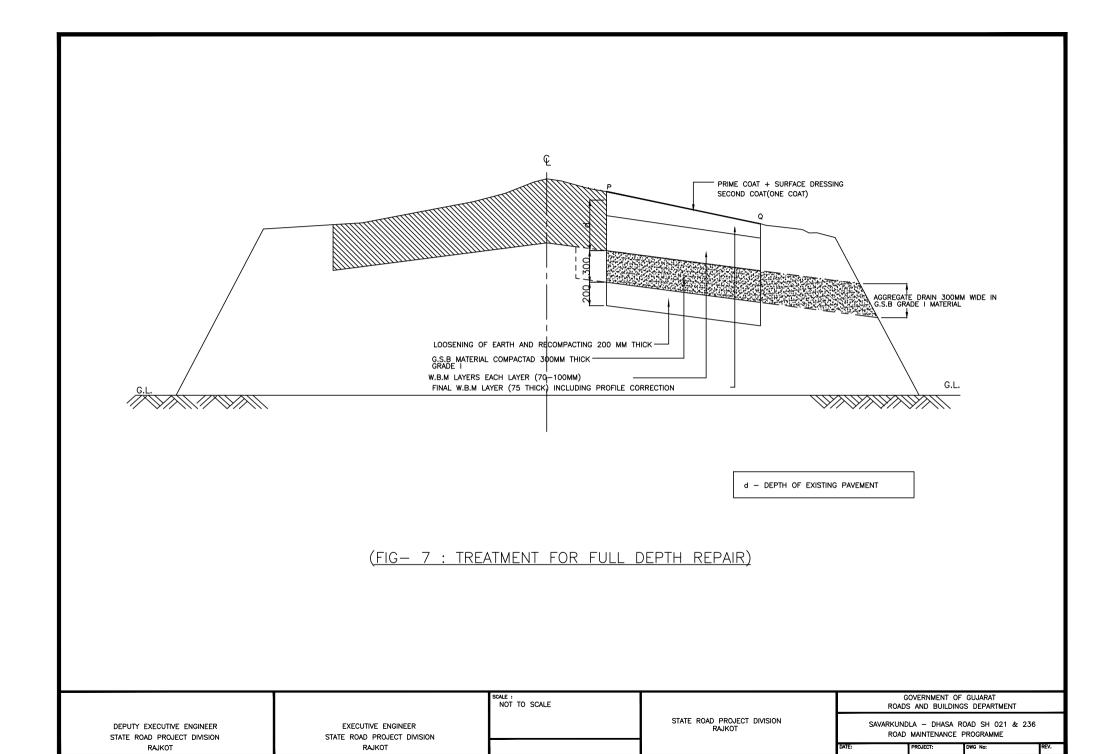
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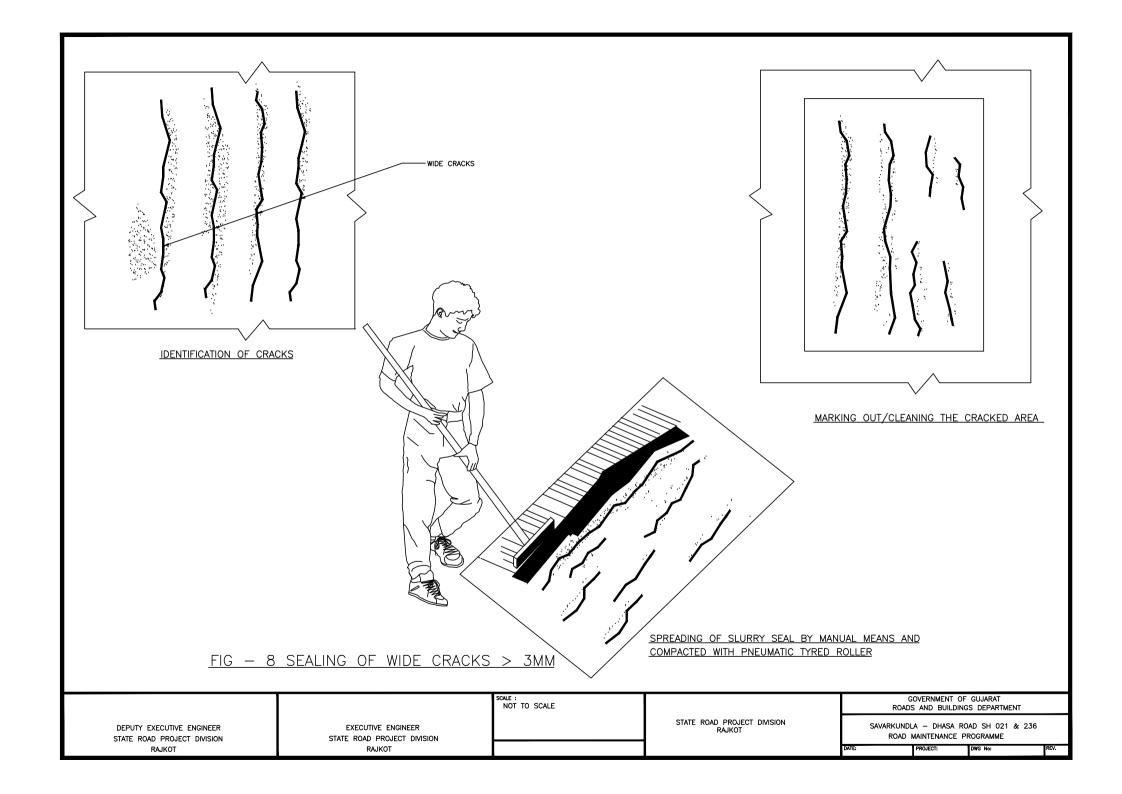
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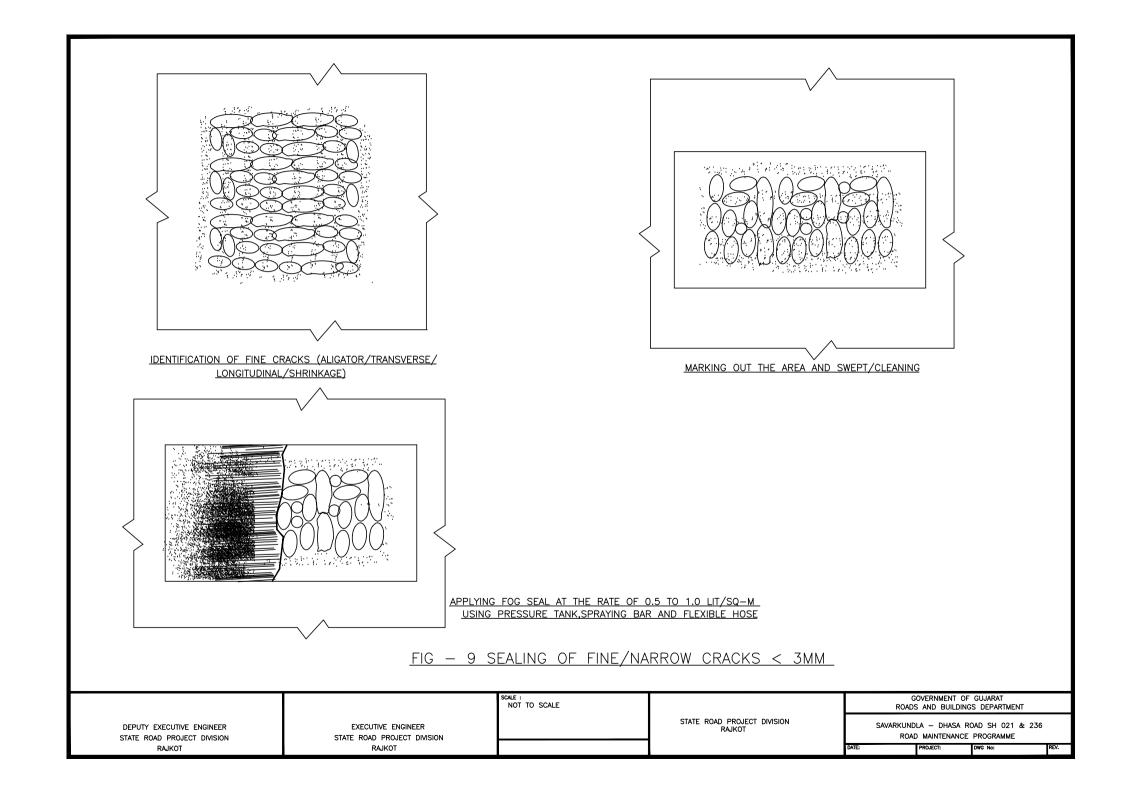
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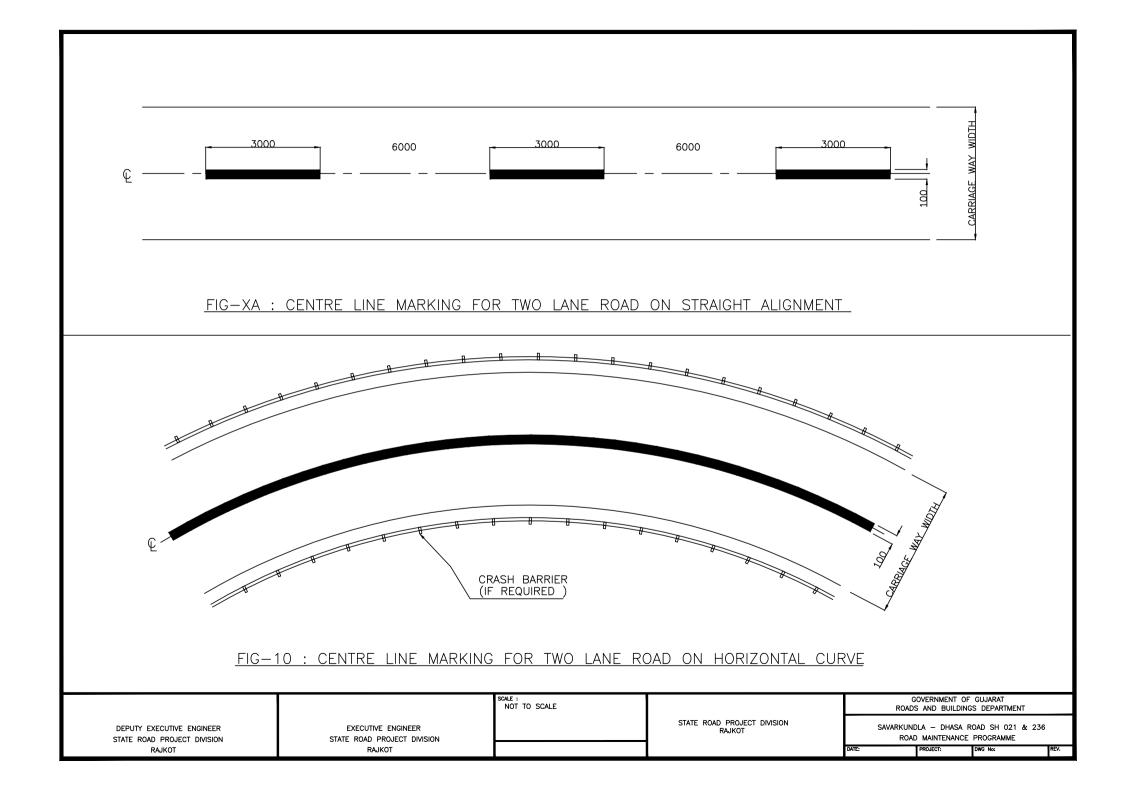
DATE: PROJECT: DWG No: REV.

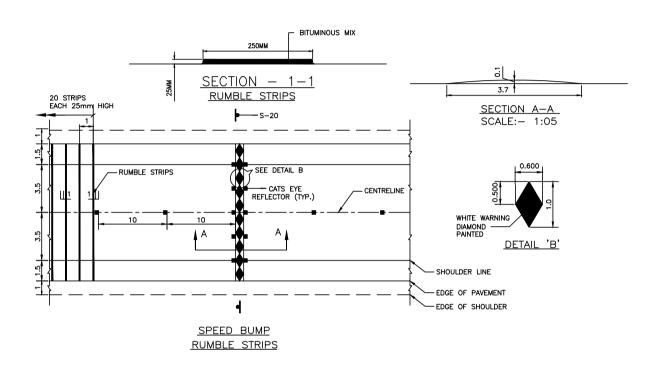












NOTES:
1. ALL DIMENSIONS ARE IN METER UNLESS OTHERWISE SPECIFIED.
2. SHOULDER WITH VARIES AS PER TYPICAL SECTIONS.

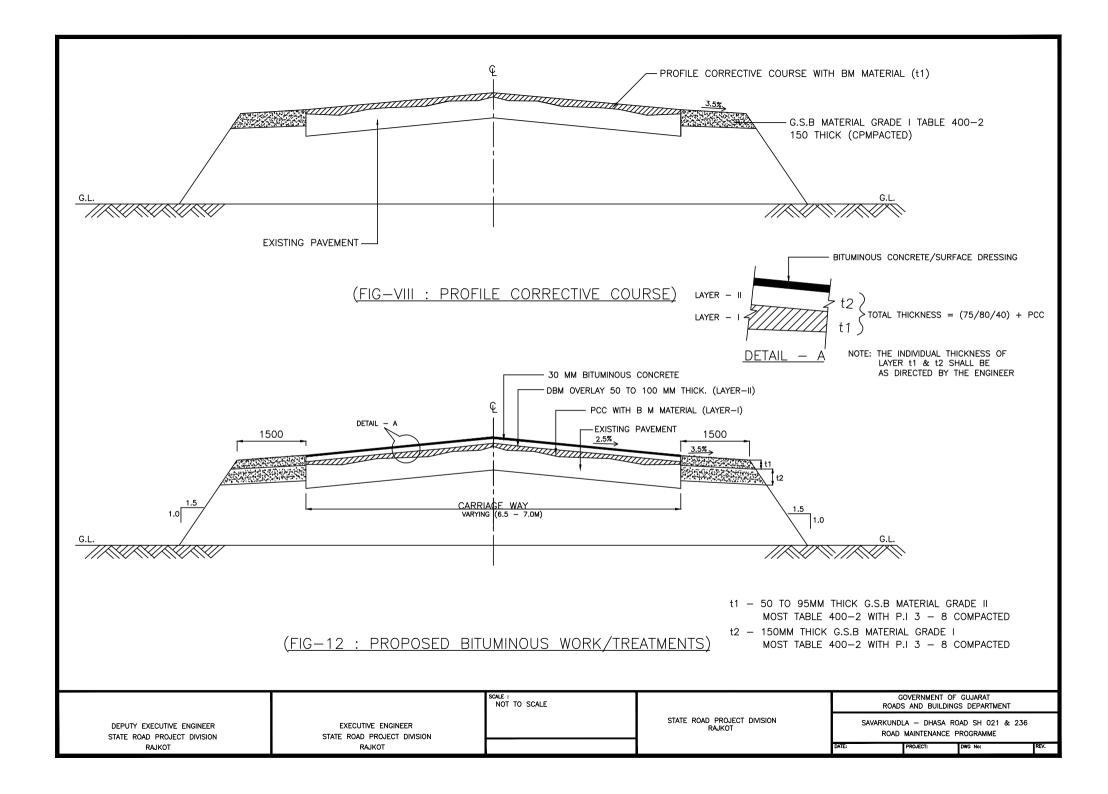
FIG - 11

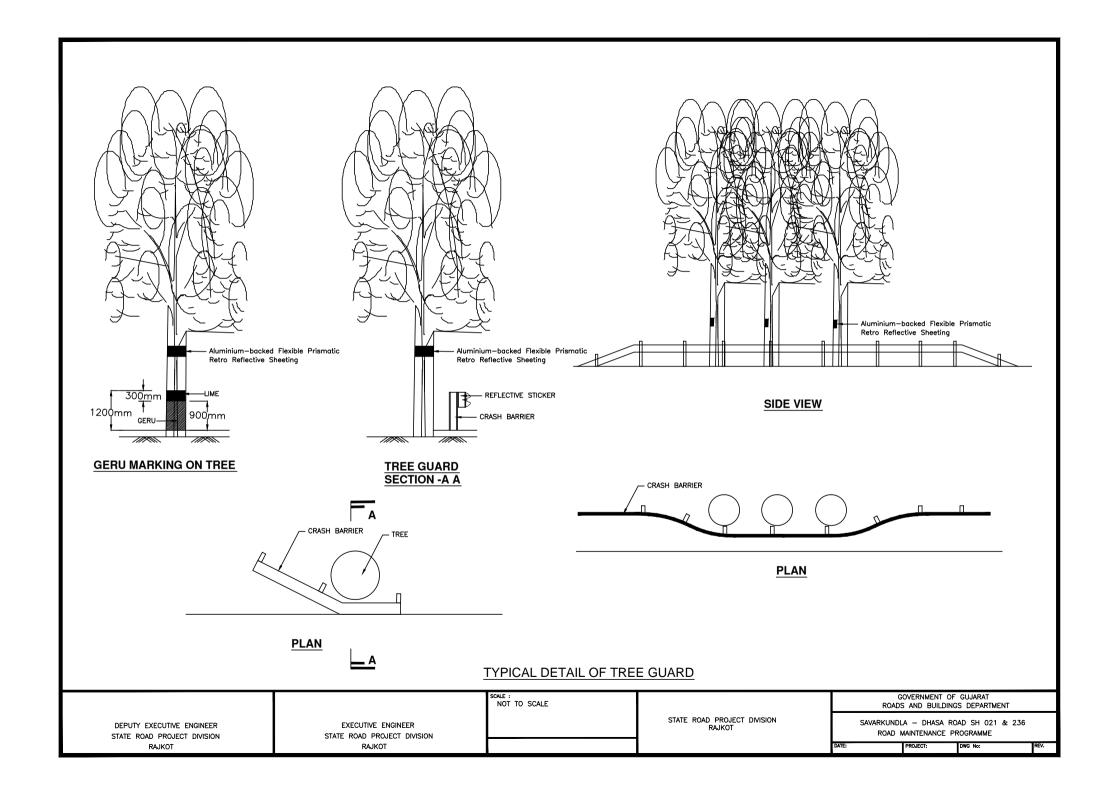
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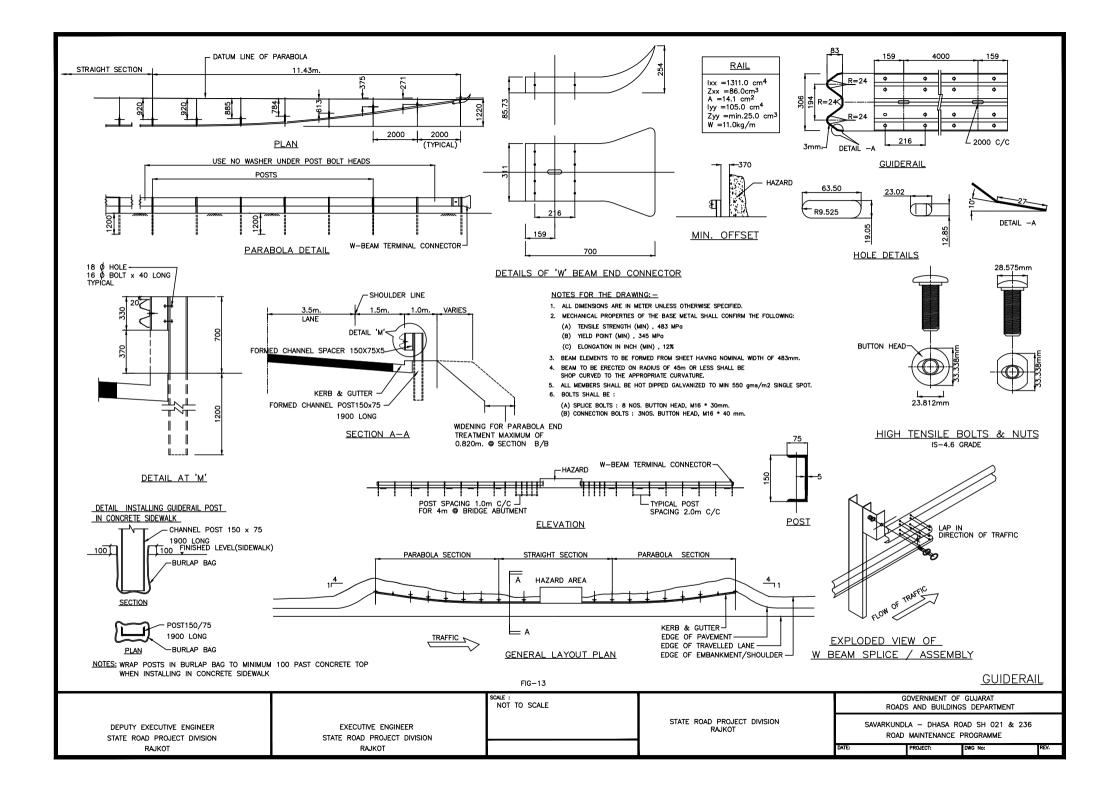
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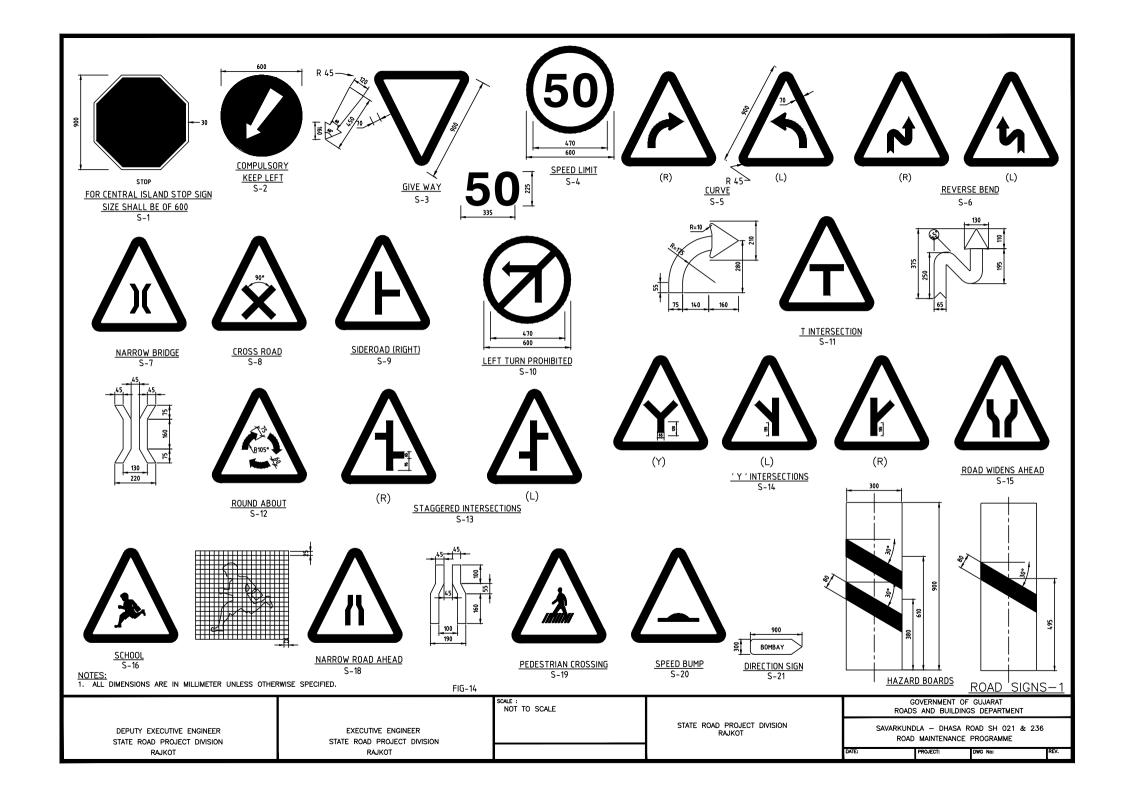
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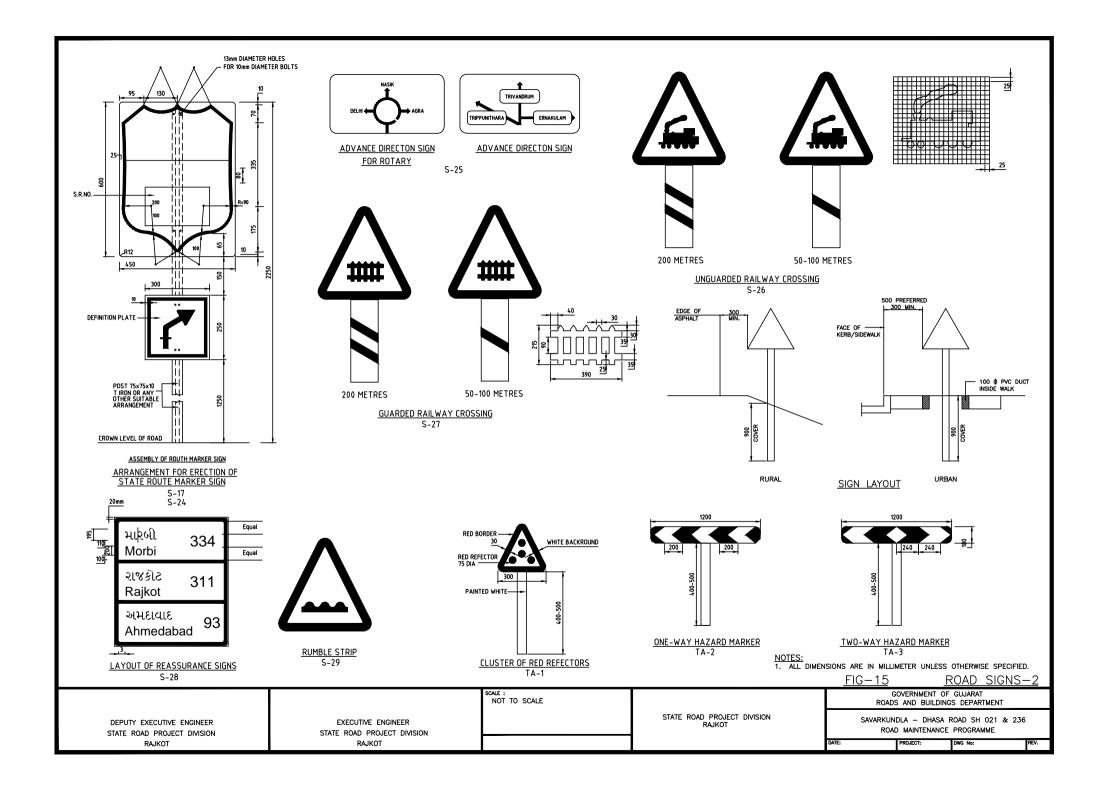
GOVERNMENT OF GUJARAT ROADS AND BUILDINGS DEPARTMENT SAVARKUNDLA - DHASA ROAD SH 021 & 236 ROAD MAINTENANCE PROGRAMME

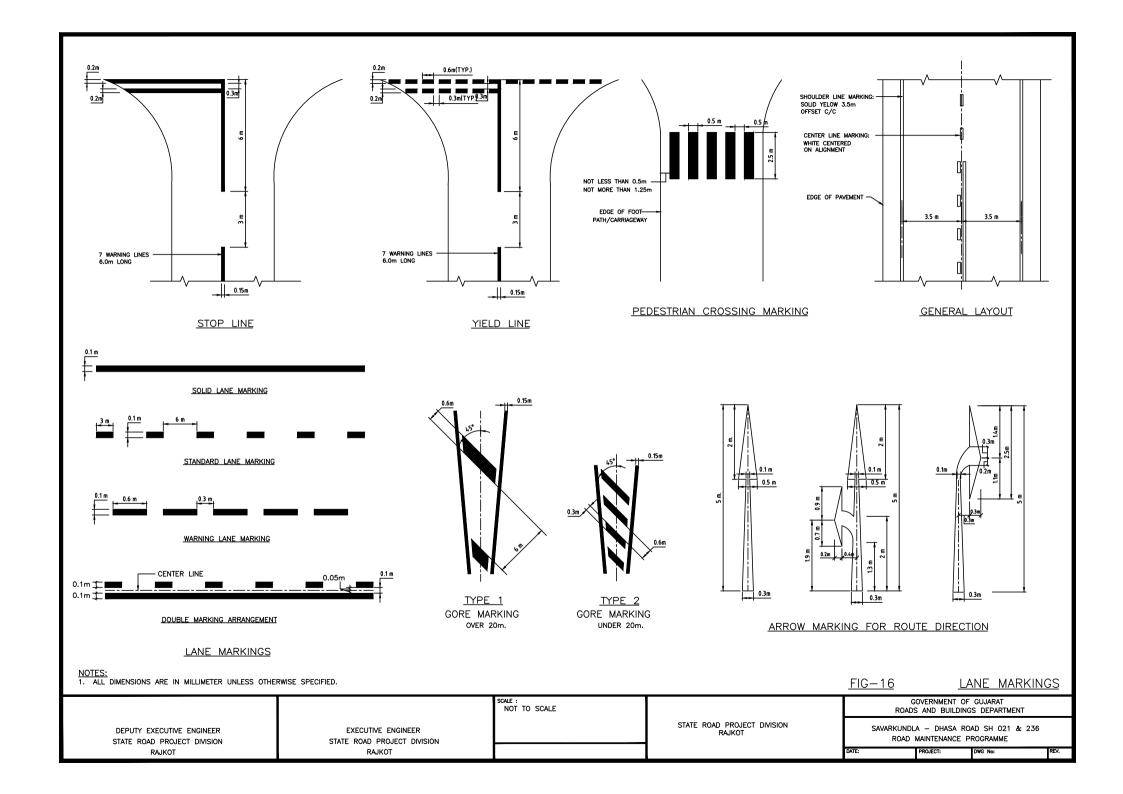


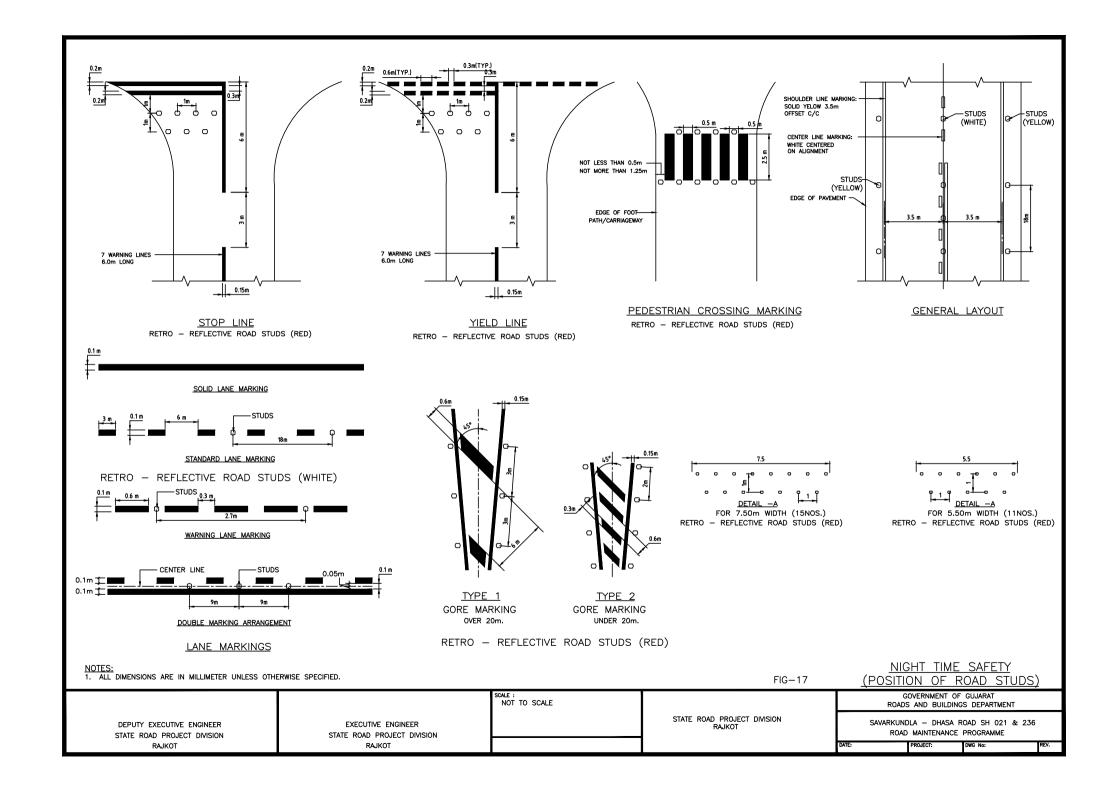


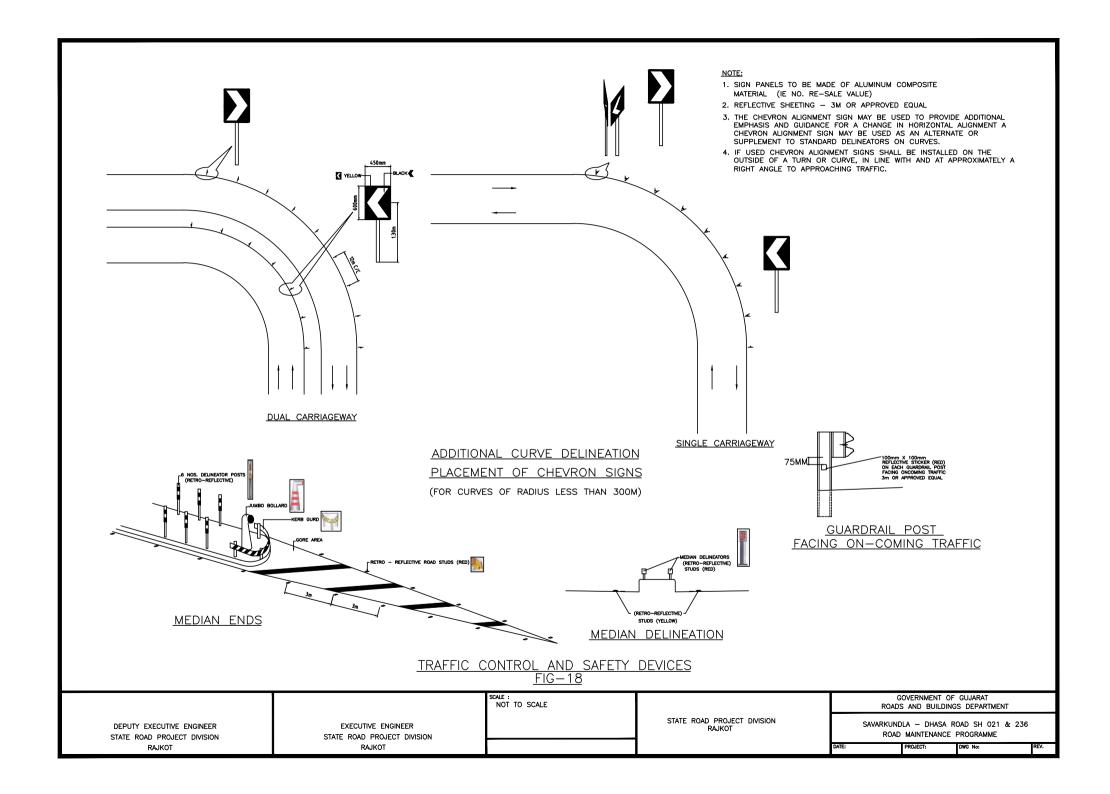


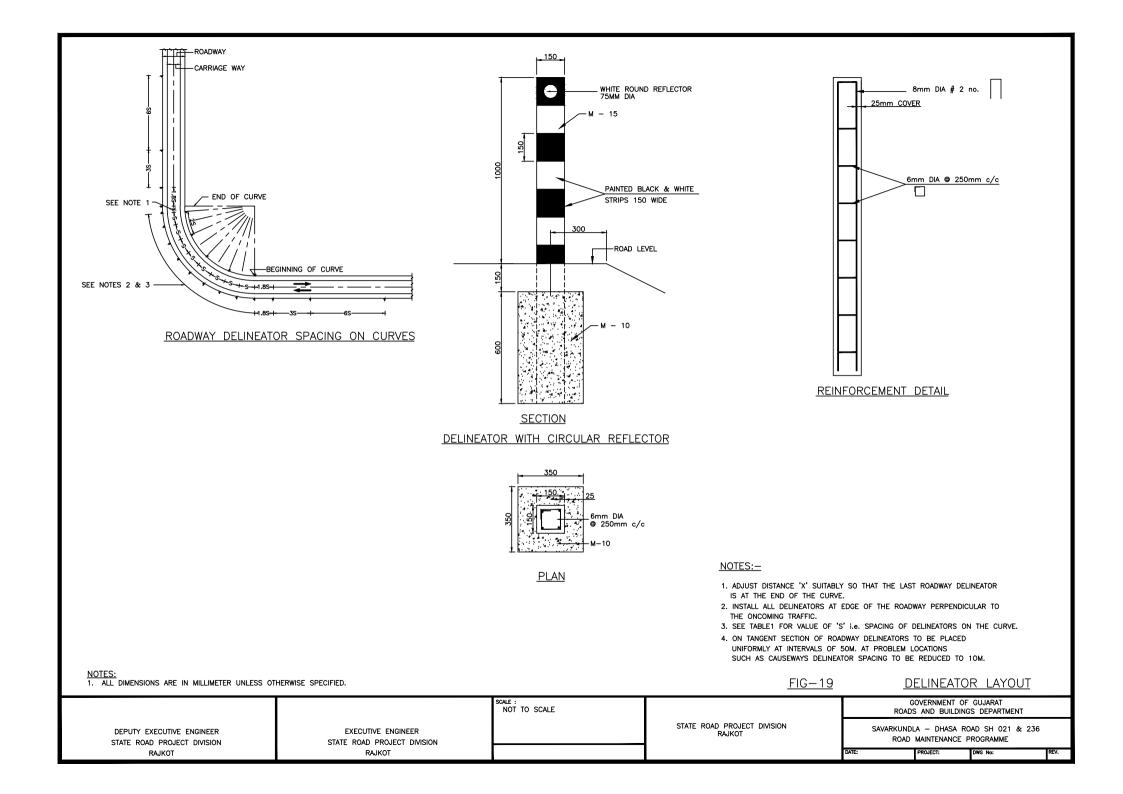












SAVARKUNDLA - DHASA ROAD SH - 236 & SH - 021 SCHEDULE

RUMBLE SRIPS			
Sr. No	Location Km Chainage	Nos	Remarks
	SH - 236		
1	1+010	2	Nr Settlement
2	5+750	2	Curveture
3	10+600	2	Village (Bhuva)
4	12+300	2	Curveture
5	26+850	2	Village (Sanalia)
6	27+800	2	Curveture
7	28+500	2	Curveture
8	32+650	2	Village (Khara) & Cross road of T-junction
9	37+400	2	Village (Gundran)
10	40+000	2	Curveture
11	40+300	2	Village (Eklera)
12	43+450	2	Both side of Raised Pedstrain Crossing (Village Ingorala & School)
13	47+300	2	Village (Hawtad)
14	48+600	2	Curveture
15	49+340	2	Village (Damnagar)
	SH - 021		
1	3+000	2	Nr Settlement
2	5+100	2	Village (Navagam)
3	9+700	2	Village (Dahithara)
4	12+200	2+2	Damnahar Nr 4-arm junction
	Note : Also at all side roads rumbler is proposed (Total 10 side roads)		

RAISED PEDESTRIAN CROSSING			
Sr. No	Location Km Chainage	Nos	Remarks
	SH - 236		
1	10+600	2	Village (Bhuva)
2	26+850	2	Village (Sanalia)
3	32+650	2	Village (Khara)
4	37+400	2	Village (Gundran)
5	40+300	2	Village (Eklera)
6	43+450	2	Village (Ingorala)
7	47+300	2	Village (Hawtad)
	SH - 021		
1	5+100	2	Village (Navagam)
2	9+700	2	Village (Dahithara)

	CRASH BARRIER		
Sr. No	Location Km Chainage	Length in RMT	Remarks
1	Nr 5+700	50	RHS & LHS
2	27+200 to 27+275	75	LHS
3	27+275 to 27+375	100	RHS & LHS
4	27+500	4x30	At all four ends of MJBR
5	32+117	55	RHS & LHS

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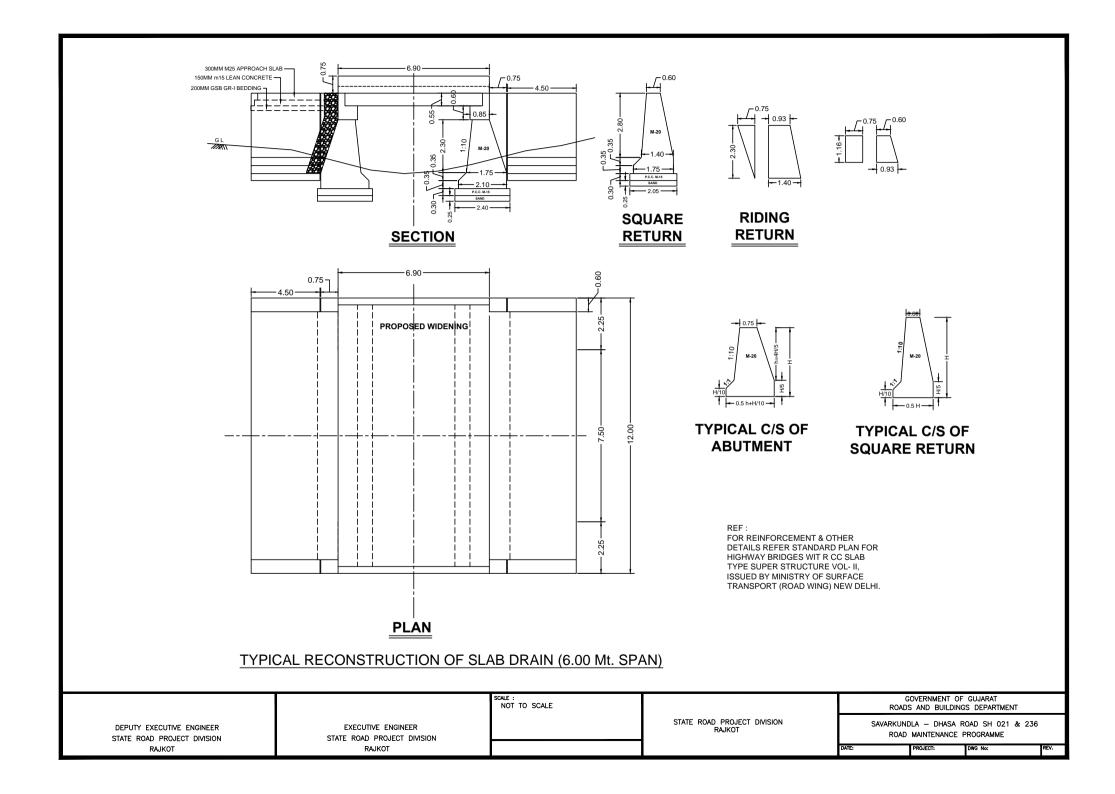
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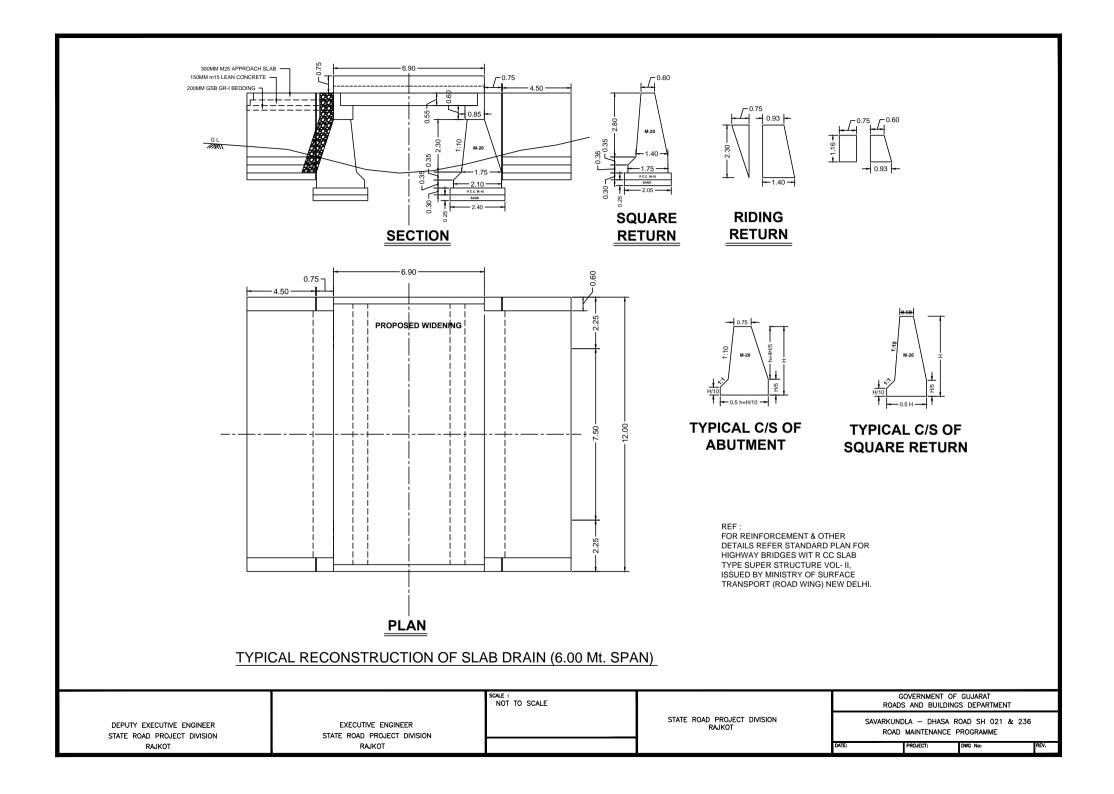
STATE ROAD PROJECT DIVISION
RAJKOT

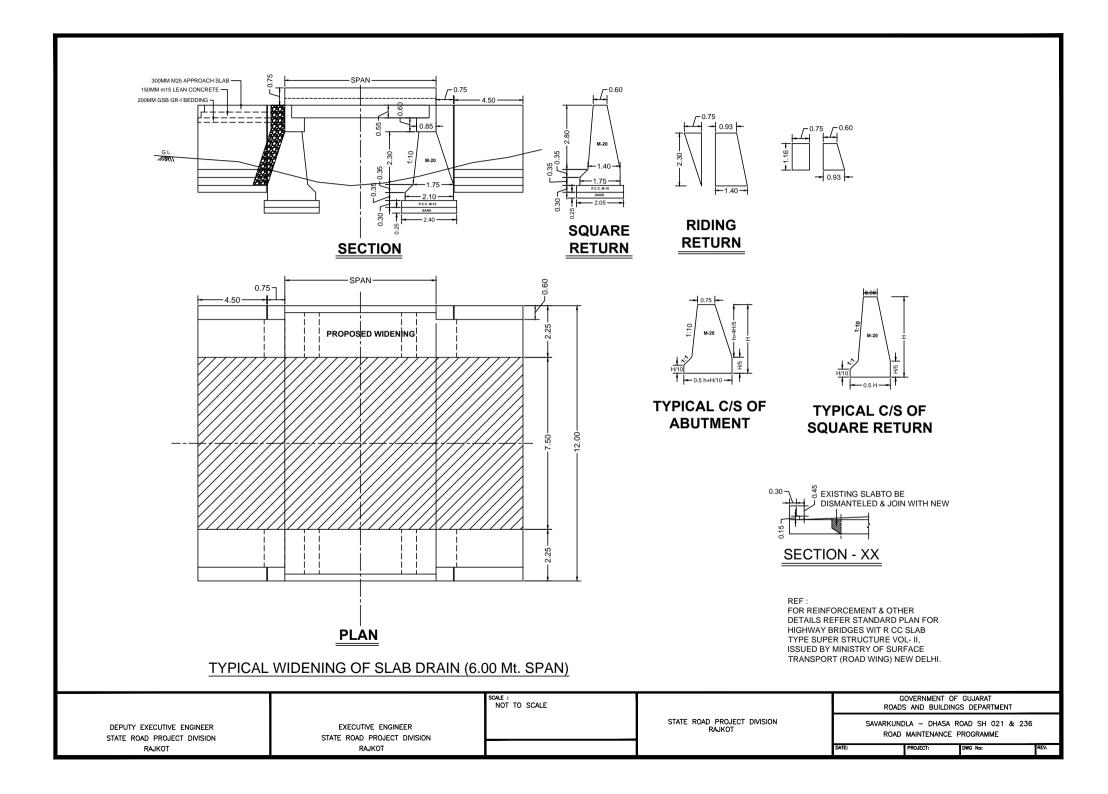
SAVARKUNDLA – DHASA ROAD SH 021 & 236
ROAD MAINTENANCE PROGRAMME

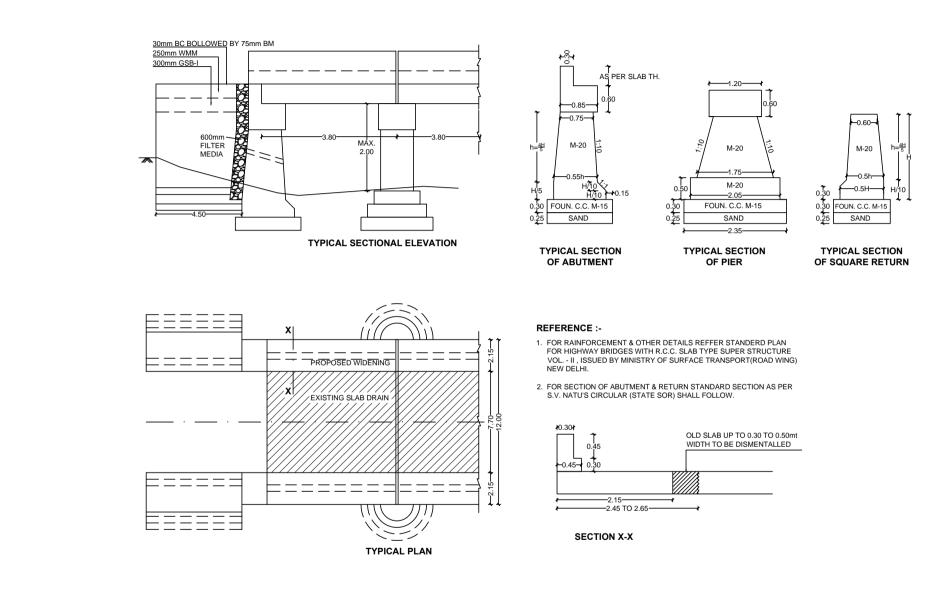
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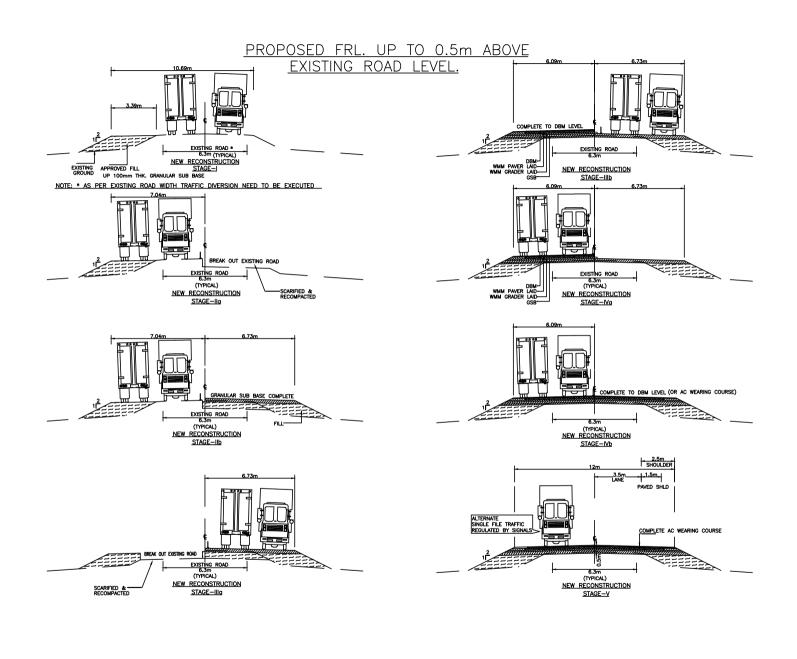
TYPICAL DRG. OF WIDENING OF SLAB CULVERTS (C/C SPAN 3.80mt & HEIGHT FROM G.L. UP TO 2.00mt)

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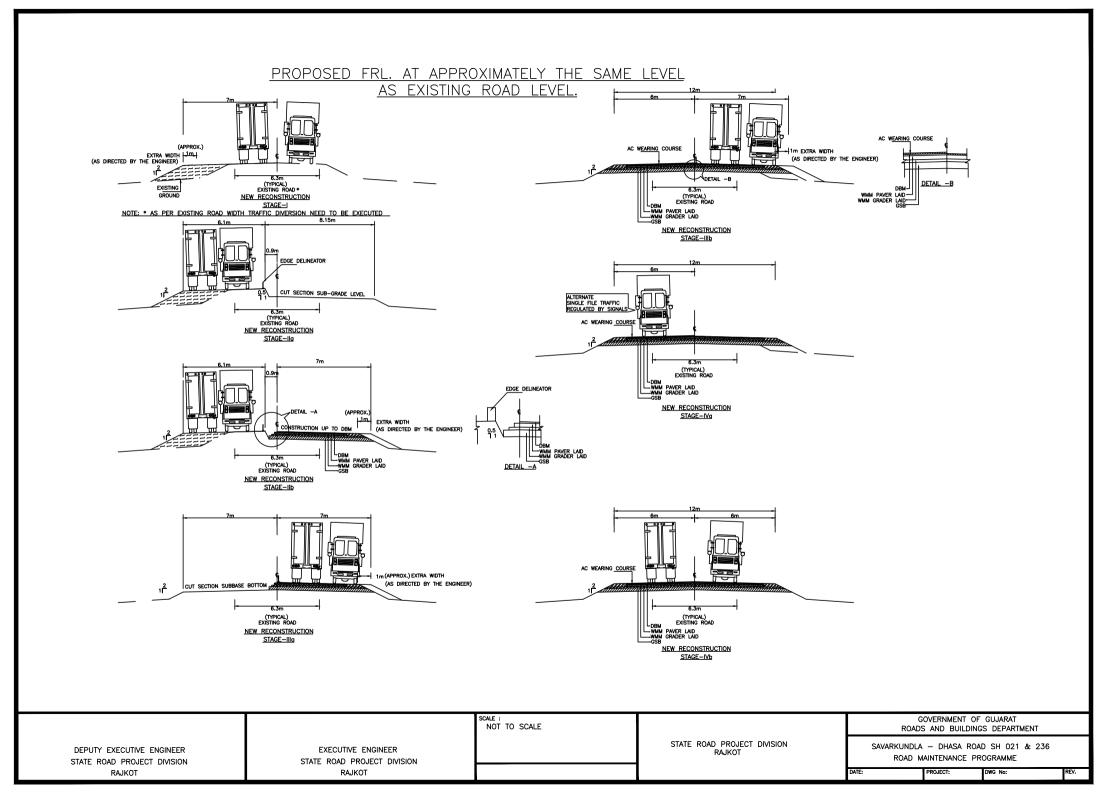
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GOVERNMENT OF GUJARAT ROADS AND BUILDINGS DEPARTMENT

SAVARKUNDLA - DHASA ROAD SH 021 & 236 ROAD MAINTENANCE PROGRAMME

ATT: PROJECT: DWG No:



Traffic Management requirements and construction methodology

Contractor's Submission

The Contractor shall submit, for the Engineer's approval, Traffic Management Plans and associated method statements at least 5 working days prior to commencement of the works. The Traffic Management Plans and method statements shall include the following minimum information and details:

- Proposed location and sequence of sub-sections for construction.
- Proposed staging of "half-width" by "half-width" road construction and traffic safety and control.
- Details of transitions to maintain safe traffic flow between various road construction zones.
- Details of temporary diversions in accordance with Specification Clause 112.3.
- Typical details of arrangements for construction under traffic including details of traffic arrangements after the cessation of work each day.

Special consideration shall be given in the Traffic Management Plans to the safety of pedestrians and workers, both by day and at night. Temporary diversions will be constructed only with the approval of the Engineer.

In general the contractor must plan his works in consideration of the following basic principles:

- Portial pavement construction over long lengths shall not be permitted. The contractor should concentrate his activities over sections such that he can complete continuous fronts of up to a maximum of say 10km before starting the adjacent front. (10km is taken as a reasonable guide.) The contractor may open more than one continuous 10km front provided that he has the separate resources to do so. The resources working on a 10km front may not be shifted to next front until no longer required on previous front. It is acknowledged that as one front nears completion plant will become available to work on a successive front. This implies some inevitable overlap between one front and its successor, which is acceptable.
- 2. For road widening operations, excavation adjacent to the existing road shall not be permitted on both sides simultaneously. Earthworks must be completed to the level of the existing road before excavation work on the opposite side will be permitted.
- 3. The construction operations taking place on a particular front must be managed efficiently such that delays between successive pavement layers are minimized.
- 4. Before the start of the monsoon season (June) the contractor shall ensure that the payement over all the fronts is complete, full width, at least to DBM level, but preferably with AC wearing course. The contractor should not start any sections of payement that he cannot complete before the start of the monsoon season.
- 5. In the absence of permanent facilities, temporary drainage and erosion control measures, as required by the Specifications, are to be implemented prior to the onset of the monsoon.
- 6. Each project road has its own unique conditions and therefore the above traffic management concept should be modified to suit, but respecting the basic principle of completion of discrete sections.
- 7. Works on CD structures requiring diversions should be sequenced in with the overall traffic management plan, but can be treated independently of the established 10km working fronts where separate resources are available. 8. When separate traffic diversions are required for CD works and other situations that demand them the minimum requirements of Spec Cl 112.3 of MORTH should be equaled or bettered. Such diversions should have smooth connections to the road with well-established and clear signage to give all required information to the road user. Such diversions should be maintained throughout their required duration.
- 9. In general traffic management and safety measures implemented should be inspected regularly by the contractor and the engineer (day and night) to rectify problems before giving cause for complaint.
- 10. The Employer is highly concerned about the quality of traffic management and safety as an integral part of the project and will not compromise on these aspects at any stage.

Separate traffic diversions as per Spec. Cl. 112.3 of MORTH shall in general be required for the following construction situations as determined by the Engineer:

- For CD reconstruction
- For CD widening if considered necessary by the Engineer
- For new roadworks when the FRL is substantially above existing RL
- For new roadworks when the FRL is substantially below the existing RL such that a separate diversion is required in the judgment of the engineer

The above criteria may be adjusted as decided by the Engineer if alternative methodology is possible and cost-effective as per actual site conditions. The above criteria may also be adjusted when the design centerline is significantly offset with respect to the existing.

In cases where separate traffic diversions are not essential or cost effective the construction methodology should be in accordance with the following guidelines :

The contractor working on a 10km section, the pavement construction (except new alignments) should be limited to 500m sub-sections with a minimum of 1 to 1.5 km between successive sub-sections to ease traffic management and safety issues. The earthworks in the widening portions are not limited in this respect. Excavation on both sides of the existing road over the same sub-section simultaneously shall not be permitted for reasons of safety to the traffic, particularly at night. Sub-sections longer than 500m may be authorized by the Engineer if two-way traffic flow can be comfortably managed and the Contractor should maintain dust control, proper road edge delineation, proper signage and traffic control. The number of sub-sections open for construction shall suit the contractor's needs to meet his approved clause 14 Programme.

Where single file traffic is permitted (only applicable to final wearing course operations), the sub-sections shall be reduced to a maximum length whereby safe traffic regulation can be physically managed. Single file traffic may not be permitted at certain locations or times of the day when traffic volumes are such that excessive congestion shall occur.

Three typical traffic management scenarios for which separate traffic diversions are not required are illustrated on the attached drawings and described as follows:

Case 1: Proposed FRL at Approximately the same level as the existing RL (Refer to drawing TM-03)

- Traffic running normally on the existing road.
- Construct the earthworks on one side only with proper benching into the existing embankment as appropriate. Temporary additional widening by approximately 1.0 to 1.25m,as directed by the engineer.
- Construct temporary overfilling to match the level of the adjacent existing pavement.
- Install proper edge delineation and temporary road signs to suit next stage.

- Divert traffic on new partial construction and half width of existing road. Earth running surface to be kept watered periodically to control dust
- Breakout half width of the existing road on the other side. Reuse salvaged materials elsewhere or dispose as appropriate.
- Prepare cut formation as per Spec Cl 301.6 of MORTH or as otherwise directed by the Engineer.
- Construct earthworks to the design levels including the use of salvaged materials from the existing road where possible.
- Construct GSB, WMM layers and DBM to the design levels.
- DBM complete to half width. Adjust temporary delineation and signs ready for next stage.

- Divert traffic onto the new DBM and work on the other side.
- · Scarify temporary earthworks and break out remaining portion of the existing road (savage materials for reuse in earthworks of adjacent section.)
- Prepare cut formation as per Spec Cl 301.6 of MORTH.
- · Construct earthworks to the design levels including use of salvaged materials from the existing road where
- Construct GSB and WMM layers to the design levels.
- · Construct DBM complete half width (full width now complete)
- Construct AC wearing course complete half width
 Adjust temporary delineation and signs ready for next stage.

- Regulate traffic to single file flow properly controlled by signalmen while the AC wearing course is completed on the other side to complete the full width.
- Shoulders and side slopes made good and finished during this stage.
 This stage with single file traffic flow should be limited to the shortest duration possible to minimize disruption to traffic flow.

(The Engineer may allow the full road width to be open for traffic after completion of DBM (Stage III) provided that wearing course operations follow without undue delay.)

Move to the next 500m and repeat the process. The bituminous paving works should be planned in conjunction with the staging of the road base works to give a continuity of operations in accordance with the contractor's materials production and paving rate. The bituminous paving works must take place as soon as practicable

During paving operations short stretches of alternating single file traffic may be permitted during off-peak times. Traffic control for single file traffic must meet the approval of the Engineer and be diligently controlled throughout its implementation.

SCALE : NOT TO SCALE

GOVERNMENT OF GUIARAT ROADS AND BUILDINGS DEPARTMENT

SAVARKUNDLA - DHASA ROAD SH 021 & 236 ROAD MAINTENANCE PROGRAMME

PROJECT:

DEPUTY EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION **RAJKOT**

EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION

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Case 2: Proposed FRL up to 0.5m above the existing RL(Refer to drawing TM-04)

Stage I

- Traffic running normally on the existing road.
- · Construct the earthworks on one side only with proper benching into the existing embankment as appropriate. Finish to match level of the existing road.
- Install proper edge delineation and temporary road signs to suit next stage.

Stage II

- Divert traffic on new partial construction and half width of existing road. Earth running surface to be kept watered periodically to control dust.
- · Breakout half width of the existing road on the other side. Reuse salvaged materials elsewhere or dispose
- Prepare cut formation as per Spec CI 301.6 Of MORTH. or as otherwise directed by the Engineer.
- Construct earthworks to the design levels including the use of salvaged materials from the existing road where possible.
- · Construct GSB to the design levels.
- · Adjust temporary delineation and signs ready for next stage.

Stage III

- Divert traffic onto the new GSB and work on the other side.
- · Break out remaining portion of the existing road (savage materials for reuse in earthworks of adjacent section.) and trim adjacent earthworks to the required level.
- Prepare cut formation as per Spec CI 301.6 of MORTH.
- Construct earthworks to the design levels including use of salvaged materials from the existing road where possible.
- Construct GSB and WMM layers to the design levels.
- Construct DBM complete half width. (or AC if approved by The Engineer)
- · Adjust temporary delineation and signs ready for next stage.

Stage IV

- Divert traffic onto the new DBM and work on the other side.
- Prepare surface of GSB, repair any traffic damage.
- Construct WMM and DBM. (DBM now complete to full width.)

Stage V

- Regulate traffic to single file flow properly controlled by signalmen while the AC wearing course is completed on each side to complete the full width.
- · Shoulders and side slopes made good and finished during this stage.
- This stage with single file traffic flow should be limited to the shortest duration possible to minimize disruption to traffic flow.

Case 3: Proposed FRL 0.5m to >1m above the existing RL(Refer to drawing TM-05)

Stage I

- Traffic running normally on the existing road.
- · Construct the earthworks on one side only with proper benching into the existing embankment as appropriate. Finish to match level of the existing road.
- Install proper edge delineation and temporary road signs to suit next stage.

Stage II

- Divert traffic on new partial construction and half width of existing road. Earth running surface to be kept watered periodically to control dust.
- Breakout half width of the existing road on the other side. Reuse salvaged materials elsewhere or dispose as appropriate.
- Prepare cut formation as per Spec Cl 301.6. or as otherwise directed by the Engineer.
- Construct earthworks to the design levels including the use of salvaged materials from the existing road where possible.
- · Construct GSB to the design levels.
- Protect half-width construction edge with sandbags or other approved method.
- Adjust temporary delineation and signs ready for next stage.

Stage III

- Divert traffic onto the new GSB and work on the other side.
- Break out remaining portion of the existing road (savage materials for reuse in earthworks of adjacent
- section.) and trim adjacent earthworks to the required level.
- Prepare cut formation as per Spec CI 301.6 of MORTH
- Construct earthworks to the design levels including use of salvaged materials from the existing road where possible
- Construct GSB and WMM layers to the design levels.
- Construct DBM complete half width. (or AC if approv ed by The Engineer)
- Adjust temporary delineation and signs ready for next stage.

- Divert traffic onto the new DBM and work on the other side.
- Construct WMM and DBM. (DBM now complete to full width.)

- Regulate traffic to single file flow properly controlled by signal men while the AC wearing course is completed on each side to complete the full width.
- Shoulders and side slopes made good and finished during this stage.
- This stage with single file traffic flow should be limited to the shortest duration possible to minimize disruption to traffic flow.

For all the above construction scenarios there shall be a need for a considerable input into planning and implementing the different stages in a practical and safe manner. There shall correspondingly be a need for clear and explicit signage correctly positioned to give adequate warning and guidance to road users. The contractor shall pay particular attention to the control of dust during the trafficking of earthen or granular surfaces and proper delineation of the edges of the traveled way. Watering to control dust should be carried out at least three times per day, if the Contractor fails to control dust from the trafficking of earthen or aranular surfaces the Engineer will instruct the Contractor to apply a temporary bituminous surface treatment at the Contractor's cost. The Contractor is responsible for maintaining and making-good any surface under trafficking to the approval of the Engineer and at no additional cost to the Employer.

The contractor may propose alternative traffic management systems provided that the concept of sectional completion and contained working sub-section are respected.

Other Traffic Management Considerations during Construction

During any particular stage of traffic management, the Contractor shall make sure that adjoining properties and access roads are not cut off unless there are viable alternatives available. Where required, the Contractor shall construct and maintain temporary access respecting all necessary safety requirements. Bus stops shall be preserved and adjusted to suit the traffic management staging with proper and safe accessibility for pedestrians.

The Contractor must train his personnel who are assigned for the purpose of traffic control and safety. These personnel must understand the importance of their role and have a proper awareness of the concept of safe traffic management. Such personnel should themselves be road users to understand safe traffic control procedures.

Safety Requirements

The importance of proper safety measures cannot be over-emphasized. It is the sole responsibility of the Contractor to implement and maintain all necessary safety measures during the course of the works. The Contractor shall adhere to the requirements of the MOST Specifications and the latest IRC codes, particularly IRC:SP:55-2001 concerning safety in road construction zones. The Engineer shall strictly monitor the Contractor's performance in the execution of his duties with respect to safety and The Engineer shall exercise his authority under the Contract to have any deficiencies remedied.

REFERENCE DRAWINGS

TYPICAL CROSS SECTIONS FOR TRAFFIC MANAGE	GEMENT CASE-1	DWG.NO.	TM-03
TYPICAL CROSS SECTIONS FOR TRAFFIC MANAGE	GEMENT CASE-2	DWG.NO.	TM-04
TYPICAL CROSS SECTIONS FOR TRAFFIC MANAGE	GEMENT CASE-3	DWG.NO.	TM-05
TYPICAL PLAN FOR TRAFFIC MANAGEMENT SEQUENCE DURING CONSTRUCTION		DWG.NO.	TM-06
SCHEMATIC PLAN FOR TRAFFIC MANAGEMENT E SUB-SECTION AT DIFFERENT STAGES	BETWEEN	DWG.NO.	TM-07
TYPICAL ARRANGEMENT OF TRAFFIC CONTROL FOR 500M SUB-SECTION	DEVICES	DWG.NO.	TM-08
TYPICAL ARRANGEMENT OF TRAFFIC DIVERSION	AT NEW CULVERT	DWG.NO.	TM-09

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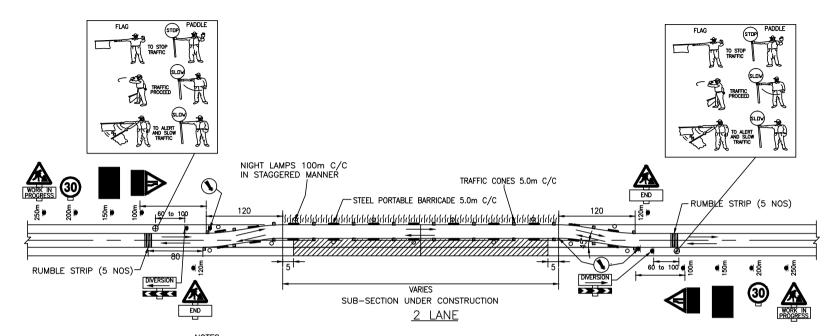
ROADS AND BUILDINGS DEPARTMENT

SAVARKUNDLA - DHASA ROAD SH 021 & 236 ROAD MAINTENANCE PROGRAMME

GOVERNMENT OF GUJARAT

EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION RAJKOT

DEPUTY EXECUTIVE ENGINEER STATE ROAD PROJECT DIVISION BAJKOT



SIGN & DELINEATORS INVENTORY AT 2 LANE

SPEED LIMIT SIGN 2 NOS. MEN AT WORK SIGN & BOARD 2 NOS. MEN AT WORK SIGN & BOARD 2 NOS. DIVERSION BOARD & SIGN 2 NOS. TRAFFIC CONES 5.0m C/C 3 NOS. KEEP RIGHT 5.0m C/C STEEL PORTABLE BARRICADE

SINGLE FILE TRAFFIC

TRAFFIC CONTROL AHED

2 NOS.

2 NOS.

- A QUALIFIED PERSONNEL AT LEAST AVERAGE INTELLIGENCE, BE MENTALLY ALERT AND GOOD IN PHYSICAL CONDITION BE SELECTED.
- FIRAGE CONTINUED BE SELECTED.

 FIRAGEM SHOULD BE EQUIPPED WITH YELLOW HELMET WITH GREEN REFLECTIVE STICKER FIXED AROUND AND REFLECTIVE JACKET ALONG WITH HAND SIGNALING DEVICES SUCH AS FLAGS AND SIGN PADDLES. RED FLAGS, STOP, SLOW PADDLES AND LIGHTS ARE USED IN CONTROLLING TRAFFIC THROUGH WORK AREA.

RED FLAG — MINIMUM SIZE $600\,$ X $600\,$ MM (POLYESTER CLOTH ADVISABLE) SECURELY FASTENED TO A STAFF OF LENGTH APPROX. 1 M

STOP SIGN PADDLE - SHAPE - OCTAGONAL (LIGHT IN WEIGHT) WIDTH - $600\,$ MM WITH RIGID HANDLE. BACKGROUND COLOR - RED, LETTER COLOR - WHITE

SLOW SIGN PADDLE - SHAPE - OCTAGONAL (LIGHT IN WEIGHT) WIDTH - 600 MM WITH RIGID HANDLE. BACKGROUND COLOR - YELLOW, LETTER COLOR - BLACK, BORDER COLOR-BLACK.

- 3. FLAGMEN NEED TO MAINTAIN THE FLOW OF TRAFFIC CONTINUOUS PAST A WORK ZONE AT RELATIVELY REDUCED SPEEDS BY SUITABLY REQULATING THE TRAFFIC. HE SHALL STOP THE TRAFFIC FOR A SHORT WHILE WHENEVER REQUIRED (E.G. FOR ENTRY AND EXIT OF CONSTRUCTION EQUIPMENT IN TO WORK ZONE).
- COUSTRUCTION EQUIPMENT IN 0 WORK AVEX. WHERE HE IS CLEARLY VISIBLE TO APPROACHING TRAFFIC AND AT A SUFFICIENT DISTANCE TO ENABLE THE DRIVERS TO RESPOND FOR HIS FLAGGING INSTRUCTIONS. A FLAGMAN NEVER LEAVES HIS POST UNTIL PROPERLY RELEVED, THE STANDARD DISTANCE SHALL BE MAINTAINED AT 60 100 M BUT CAN BE ALTERED DEPENDING UPON THE APPROACH SPEED AND SITE CONDITIONS. IN URBAN AREAS THIS DISTANCE
- SHALL BE TAKEN AS 20 M TO 50 M.
- STANDARD SIGNALS TO BE GIVEN BY FLAG MEN AND THEY SHOULD UNDERGO SPECIAL TASK TRAINING PROGRAM.
- 7. USE TRAFFIC LED FLASH LIGHTS AT NIGHT INSTEAD OF FLAGS.

GENERAL NOTES:

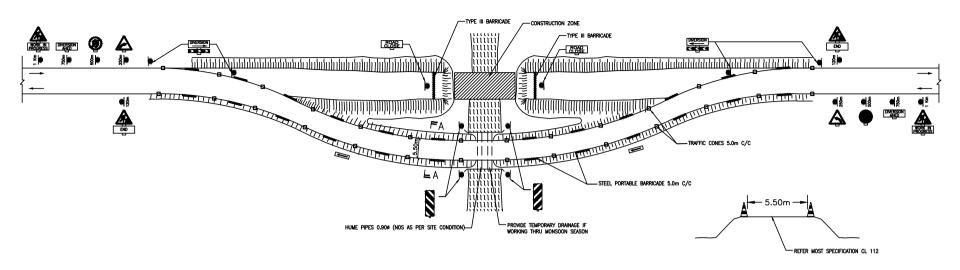
- 1. ALL DIMENSIONS ARE IN METER. UNLESS OTHERWISE SPECIFIED
- 2. CONTRACTOR SHALL SUBMIT DIVERSION PLAN FOR ENGINEER'S APPROVAL PRIOR TO CONSTRUCTION.
- 3. CONTRACTOR SHALL MAINTAIN DIVERSION THROUGHOUT TILL THE CONSTRUCTION IS COMPLETE.
- 4. CONTRACTOR IS RESPONSIBLE FOR THE SECURITY OF THE SIGNS AND DELINEATORS.
- 5. PROVIDING BARRICADING/CAUTION TAPES OF HIGH QUALITY PVC TAPE TUBE TYPE TO ENCLOSE CONSTRUCTION AREA.
- 6. TYPE OF RUMBLE STRIP WILL DEPEND ON ROAD SURFACE(IF SURFACE IS BLACK TOP-BITUMEN, IF GSB-GSB)
- 7. USE TWO ROWS OF ROPE LED LIGHTING WITHIN CONSTRUCTION ZONE DURING NIGHT ON BOTH EDGES OF MOTORABLE CARRIAGEWAY.

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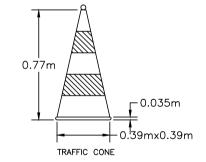
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SIGN & DELINEATORS INVENTORY

A	MEN AT WORK SIGN & BOARD	2 NOS.	
NOTE IN ARROGRESS			
END	MEN AT WORK SIGN & BOARD	2 NOS.	2.5m
DWERSTON	DIVERSION BOARD	2 NOS.	
Ø	OVERTAKING PROHIBITED SIGN	2 NOS.	0.3m 1.5m
	CURVE SIGN	2 NOS.	45x45x5mm ANGLE IRON SECTION
DIVERSION	DIVERSION BOARD & SIGN	2 NOS.	STEEL PORTABLE BARRICADE
CLOSE	ROAD CLOSED BOARD	2 NOS.	
	TYPE III BARRICADE (AS PER IRC:SP:55-2001)	2 NOS.	
	HAZARD MARKER	4 NOS.	NOTES: 1. ALL DIMENSIONS /
	TRAFFIC CONES	5.0m C/C	2. CONTRACTOR SHAI 3. CONTRACTOR SHAI 4. CONTRACTOR SHAI 5. PROVIDING BARRIC
	STEEL PORTABLE BARRICADE	5.0m C/C	5. FRAVIUM DAVIGO 6. USE TWO ROWS C EDGES OF MOTOR



- IS ARE IN METER. UNLESS OTHERWISE SPECIFIED
- HALL SUBMIT DIVERSION PLAN FOR ENGINEER'S APPROVAL PRIOR TO CONSTRUCTION.
- HALL MAINTAIN DIVERSION THROUGHOUT TILL THE CONSTRUCTION IS COMPLETE.
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- 6. USE TWO ROWS OF ROPE LED LIGHTING WITHIN CONSTRUCTION ZONE DURING NIGHT ON BOTH EDGES OF MOTORABLE CARRIAGEWAY.

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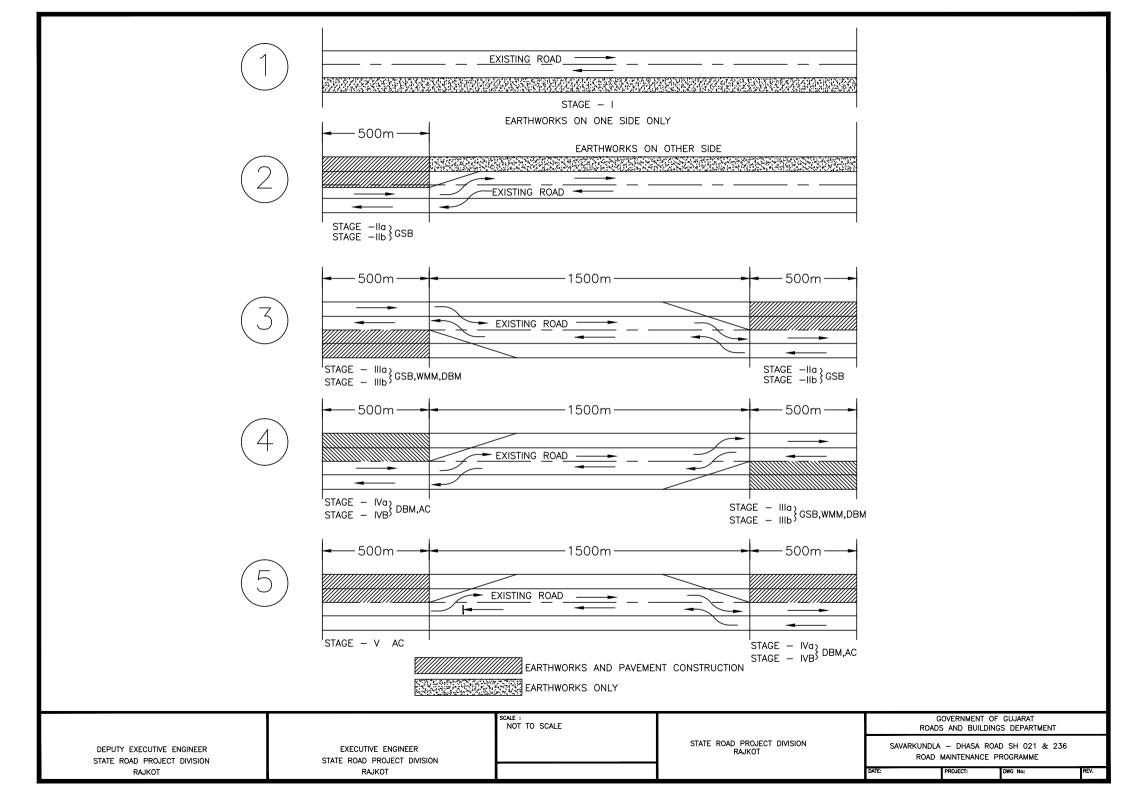
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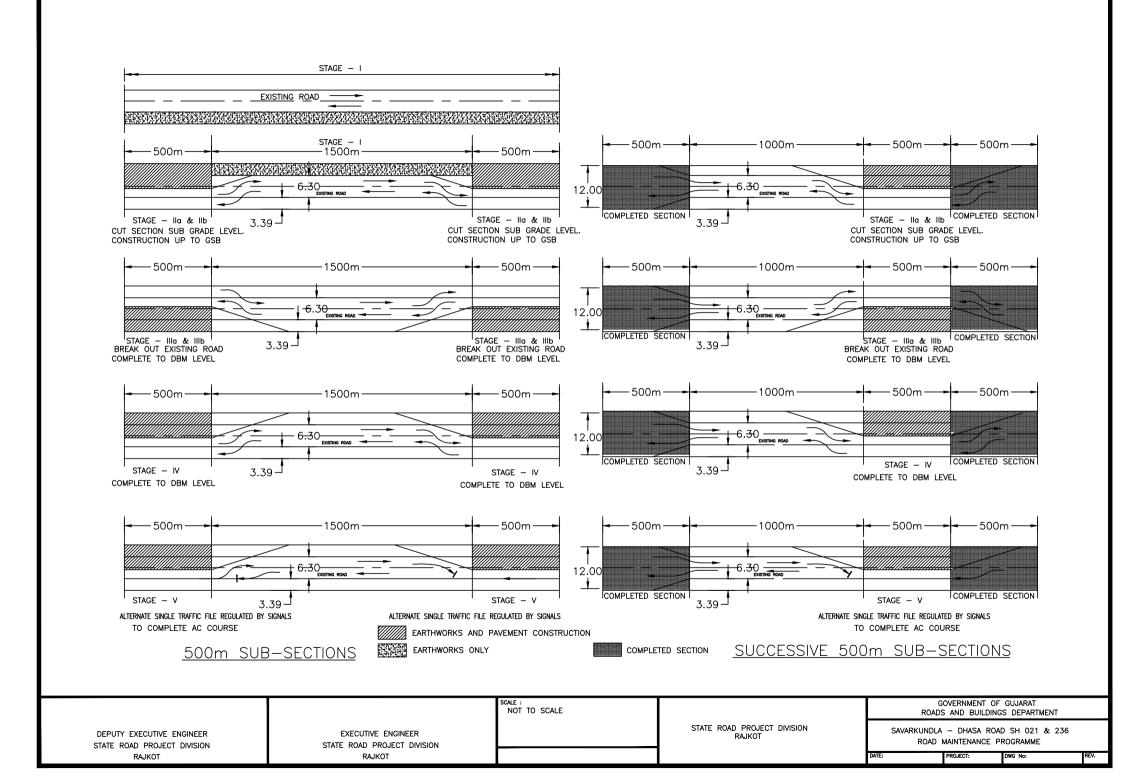
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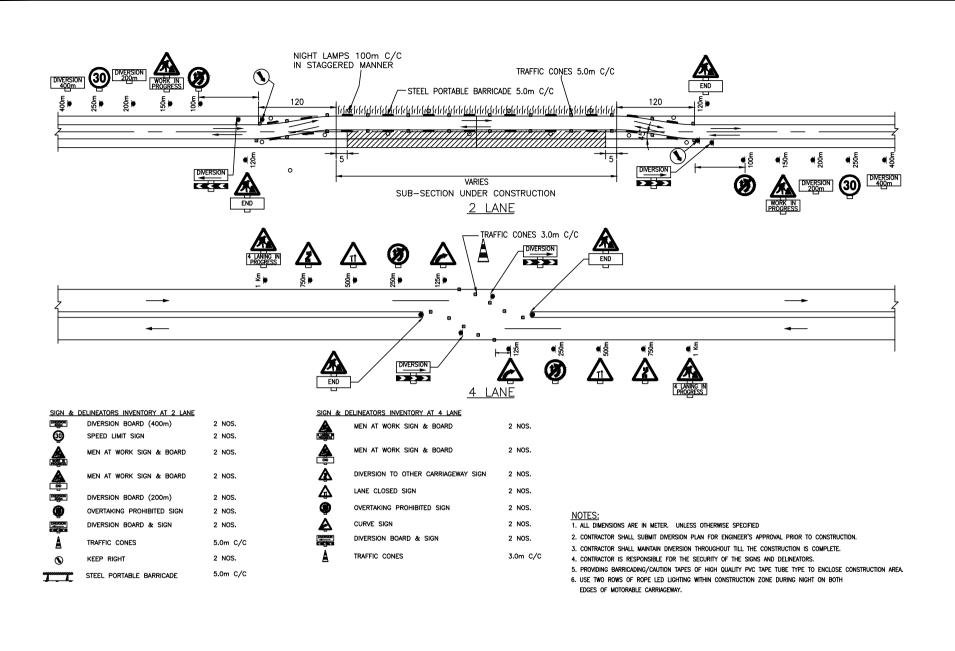
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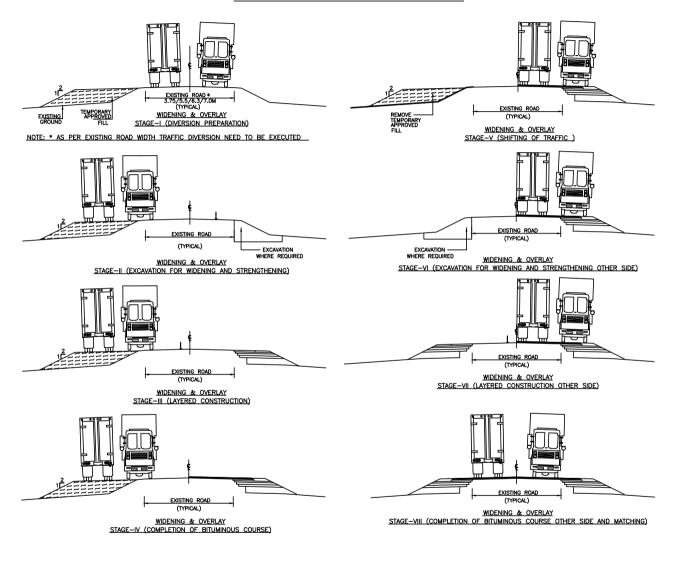
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PRO

WG No:

OVERLAY ON EXISTING ROAD



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